RADBOURNE SEVERANCE 130 MAPLE RIDGE RD. STORMWATER MANAGEMENT REPORT

Prepared for: Cherilyn Radbourne, Blair Radbourne & Dennis Radbourne 558 Punkinseed Lane Kemble, Ontario NOH 1S0

Prepared by:
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RADBOURNE SERVERANCE - 130 MAPLE RIDGE RD.

GEORGIAN BLUFFS, ONTARIO

STORMWATER MANAGEMENT REPORT

November 2021

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CD

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Natural Heritage Environmental Impact Study dated December 2020

Nitrate Study prepared by Peto MacCallum dated September 13, 2021

1. INTRODUCTION

1.1 Background

Darryl M. Robins Consulting Inc. (DMRC) has been retained by the Radbourne family to prepare a Stormwater Management Report for the purpose of obtaining a severance of their property at 130 Maple Ridge Road, in Georgian Bluffs (subject property). The property is legally described as Part of Lot 17, Concession 2. The clients wish to sever the property into 4 parcels to establish 3 residential lots and a retained farm parcel. The residential development will be limited to the most easterly portion of the property. Please refer to the Site Plan (M20005-SP and the Location Plan (Figure No. 1) for the property and development limits.

1.2 Purpose of Report

The purpose of this report is to present a preliminary assessment of the subject development's ability to provide quantity and quality controls for stormwater management.

1.3 Lot Layout

An Natural Heritage Environmental Impact Study (EIS) was completed by AWS Environmental Consulting for the client in December 2020. The EIS identified that the building envelopes (including area of all structures, septic treatment area, access laneway & servicing corridors) shall not exceed 0.4ha and a minimum retained forest environment width of 15m between forest clearing / building envelope areas. Three (3) new lots are proposed on the south side of the central watercourse. The retained portion will provide access to the lands behind the new residential lots. The complete EIS is available on the CD.

2. EXISTING CONDITIONS

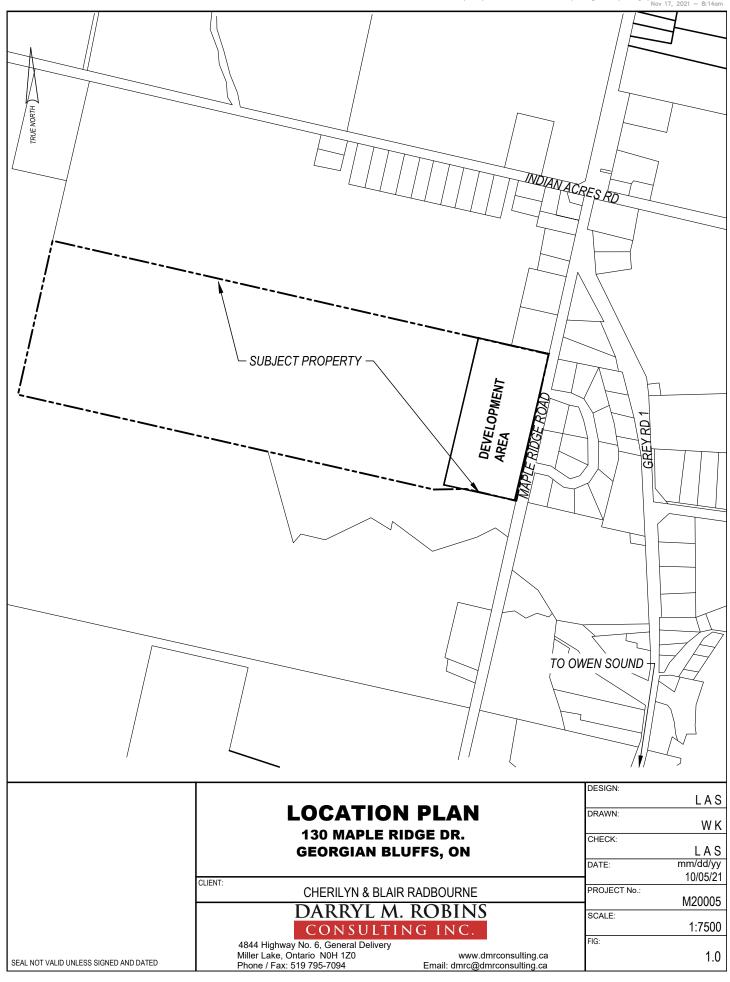
2.1 General

The subject property is located on the west side of Maple Ridge Road, in Georgian Bluffs. The existing property description is summarized below in Table 2.1 below:

Table 2.1: Property Description

_	1 7	•			
	Legal	Civic Address	Area (Ha)	Existing Use	Zoning
	Description				
ſ	Part of Lot 17,	130 Maple Ridge	31.25	Farm and Bush	PD and NEC
	Concession 2	Road			

The property is bounded by properties zoned NEC to the west north and south. Some properties to the north are also zoned R1. The properties on the east side of Maple Ridge Road are zoned R1.



The three (3) new residential lots are proposed in the area south of the central watercourse with the zoning designation of Plan Development (PD). For the purpose of this severance, it is assumed that the retained parcel will also be developed.

2.2 Topography

The topography of the property is variable with significant hills and valleys (ravines). Overall the property slopes from west to east with a slope of approximately 1%. There are areas with significant slopes of 2.5H:1V. The top of bank of the Indian Creek watercourse is located along the southern property line. There is a watercourse located in the centre of the property. An additional minor water course is located on the north side of the property. However, the overall slope of the development area is 3%.

2.3 Geology/Soils

The property is approximately 31.25 Ha. and as such, contains various soil types within the property.

The soils map entitled <u>Soils of Grey County – North Sheet – Ontario (Soil Survey Report No. 17),</u> Agriculture Canada, 1983; identifies that the development area is composed of "Osprey" (OI) and "Breypen" (Bp) depending on the location. "Breypen" soil consists of 0-12 inches of soil mantle over limestone bedrock. Numerous outcroppings and large boulders. Drainage is considered to be variable. Topography and surface stoniness is described as: Nearly Level with numerous rock outcrops very stony.

"Osprey" soil consists of 4 inches of very dark brown loam; over 2 -4 inches of brown loam over 2 inches of brown clay over grey-brown stony calcareous till. Drainage is considered to be Good. Topography and surface stoniness is described as: Irregular moderately sloping to irregular steeply sloping. Very stony.

Other soil types identified on the subject property include "Toledo" (Tc) and "Wiarton" (Ws) but these soil types are not located within the development area.

A hydrogeological assessment has been prepared within the development area by Peto MacCallum. The results of investigation are included in Appendix A. Peto MacCallum described the soil as sandy silt or silt. Groundwater was noted at depth of 2.9m.

3. STORMWATER MANAGEMENT

3.1 Existing Drainage

The subject property falls from the west to the east with a slope of approximately 1%. The Indian Creek crosses through the western portion of the property. There is significant variability in the topography of the property depending on the location. Within the development area there are a couple valleys containing

watercourses. The majority of the development area drains towards the central water course. A portion of the side drains towards the Indian creek and a portion drains to the watercourse located on the north side of the property. Please refer to the Overall Topographic Mapping Figure No. 2, which shows the overall topography of the area.

3.2 Quality Controls

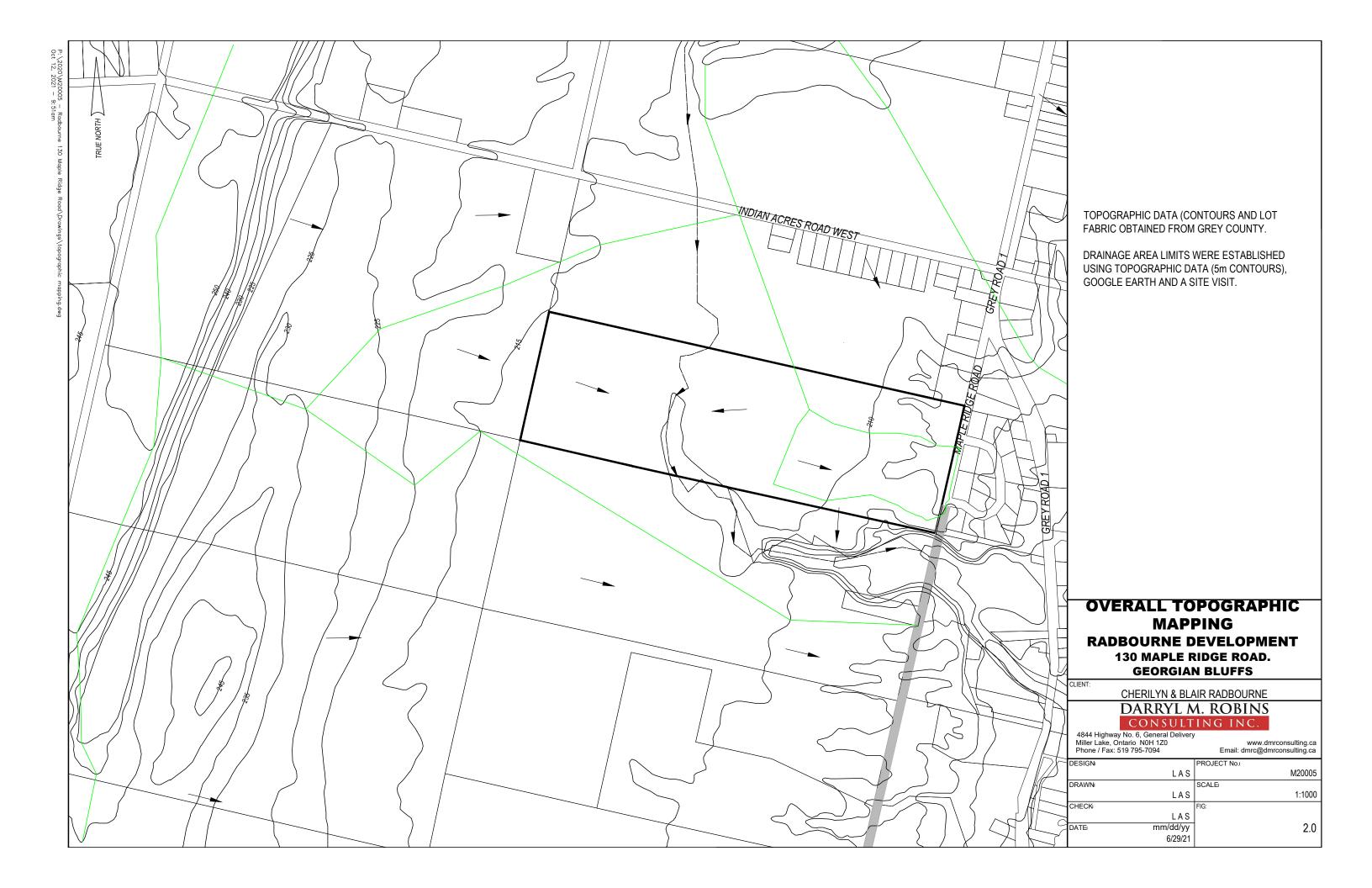
The Grey Sauble Conservation Authority (GSCA) has advised that the stormwater management report shall address quality controls. Typically, developments are required to provide 'Enhanced' level Quality Controls The "enhanced" level of protection requires that the stormwater management facilities be designed to remove 80% of the total suspended solids (TSS) on a net annual basis.

3.2.1 <u>Selection of Quality Treatment Facility</u>

In selecting a suitable stormwater management facility for this project, we have reviewed the constraints for stormwater management facilities in conjunction with Table 4.1 – "Physical Constraints of SWMP Types" of the "MOE's Stormwater Management Planning & Design Manual". A summary is provided below (see Table 3.2.1).

Table 3.2.1: Summary of SWMP Constraints

SWMP	Constraint	Requirement	Achieved
_	Topography	< 5%	variable
Redi	Soils	> 15mm/hr	N
Reduced Lot Grading	Bedrock	none	Y
	Groundwater	none	Y
	Area	none	Y
	Topography	None	Υ
Wet Pond	Soils	None	Υ
	Bedrock	None	Υ
	Groundwater	None	Υ
	Area	>5 h a	Υ
Gra	Topography	< 5%	Υ
Grassed Swale	Soils	none	Υ
d Sv	Bedrock	none	Υ
vale	Groundwater	none	Υ
	Area	< 2 ha	Υ*
<u></u>	Topography	None	Υ
/ grit	Soils	None	Υ
Oil/ grit Separator	Bedrock	None	Υ
oarat	Groundwater	None	Y
.or	Area	< 2 ha	Y



There is not sufficient space to accommodate a stormwater management Pond. Grassed swales can provide capacity to provide the quantity controls. So the Grassed Swales appear to be the most suitable option to address quality controls.

3.3 Quantitative Controls

The Grey Sauble Conservation Authority has requested that a stormwater management report be prepared to address the stormwater quantity and quality concerns. Copies of the correspondence are located in Appendix B. Autodesk's Storm & Sanitary Analysis modeling software was used to model the predevelopment and post-development conditions for the 2, 5, 10, 25, and 100 year design storms. The SCS was the Hydrology method used in the storm modeling and a six (6) hour storm was run using rainfall data for Owen Sound from the Ministry of Transportation's IDF Curve Look-up website. Please refer to the storm data and the modelling input parameters in Appendix B.

MTO Design Chart 1.02 – "Spatial Extent of Zones for Application of Regional Storms" identifies that the Timmins OR the 100-year storm is to be considered for the Regional Storm. As such, it is understood that the review of the 100-year storm will be sufficient. A copy of MTO Design Chart 1.02 is enclosed in Appendix B for easy reference.

3.3.1 Pre-Development Flows

The drainage patterns of the subject property were reviewed and the "CN" values were calculated for the drainage areas. The property appears to have 2 catchment areas. The Time of Concentration (Tc) was calculated using the Bransby-Williams Formula. Please refer to Figure 3 which identifies the predevelopment areas. The outlet in the stormwater model is the downstream culvert crossing Maple Ridge Road.

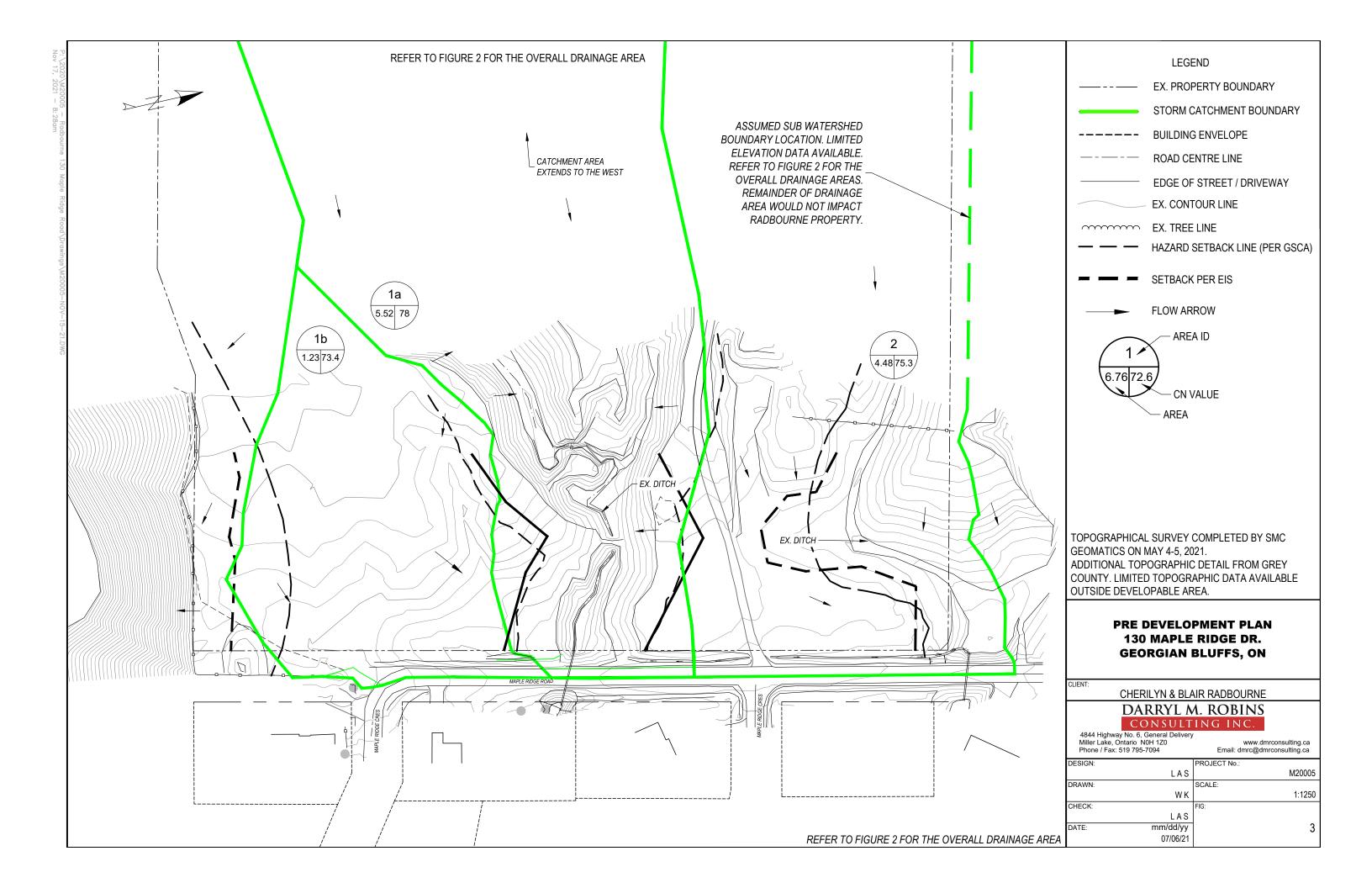
A summary of pre-development flows is provided below in Table 3.3.1. Please refer to the calculations and modelling results included in Appendix C.

Table 3.3.1: Pre-development Flows

Area	Storm Event (Design Flow - L/s)								
Alea	2	5	10	25	100				
1	1 46.79 125.98		191.27	284.1	436.3				
2	2 22.19 59.11		95.40	148.62	241.32				
Outlet	64.99	179.39	281.79	431.18	674.07				

It is noted that the 450mm dia. culvert crossing Maple Ridge Road and conveys flows from the central watercourse surcharges in the 100 Year design storm. Typically, culverts are only sized to handle the 25 year storm, so this would be considered acceptable.

^{*}The development areas are less than 2 hectares.



3.3.2 Proposed Stormwater Works

The property will be severed into four (4) parcels. Three (3) residential lots are proposed on the south side of the property subject property and one (1) parcel is to be retained for farming purposes. It is assumed that retained parcel for farming may also be developed with a dwelling. The proposed house design (size) and driveway configuration for the new lots are unknown at this time. It was assumed for the purposes of this assessment that the proposed houses/ structures would be approximately 3,227 square feet (300 square meters) and the surface area of the driveway would be approximately 110 to 190 square meters. Please refer to the Post Development Plan (Figure 4). It was assumed that the driveways would be asphalt or another impervious surface type.

It is proposed that the new lots will drain towards the existing Maple Ridge Road allowance. A minimum of 0.75m wide grassed swale is proposed within the Maple Ridge Road allowance provide quantity and quality controls. The design for grassed swale was established to comply with grassed swales design criteria in the Ministry of the Environment's Stormwater Management Planning and Design Manual dated March 2003. The guidelines identify a minimum bottom width is 0.75m with a maximum of 2.5H to 1V side slopes. Based on the existing contours it appears that there is a poorly defined watercourse or that some overland flow may occur towards the east across lot 3. It is proposed to install a grassed swale between lots 2 and 3 to redirect the surface water along the property line and not across the development area. The ultimate outlet would remain the same.

Please refer to the Grading Plan (GP1) which identifies a grassed swale with a 0.75m – 1m base width on Maple Ridge road along lots 1 to 3. Please refer to the Grading Plan (GP2) for the grassed swale with a 1m wide base width which is also proposed on Maple Ridge Road along lot 4.

3.3.3 Post-Development Flows

The development was divided into sub-catchment areas with varying run-off coefficients 74.5 – 83.4. The Time of Concentration (Tc) was calculated using the Bransby-Williams Formula for each sub-catchment area. Please refer to the Post-Development Area Plan (Fig. 4) which identifies the post-development areas and the respective run-off coefficients. Please refer to the calculations and modelling results in Appendix B.

A summary of the Post-Development Flows for each design storm is provided in Table 3.3.3.

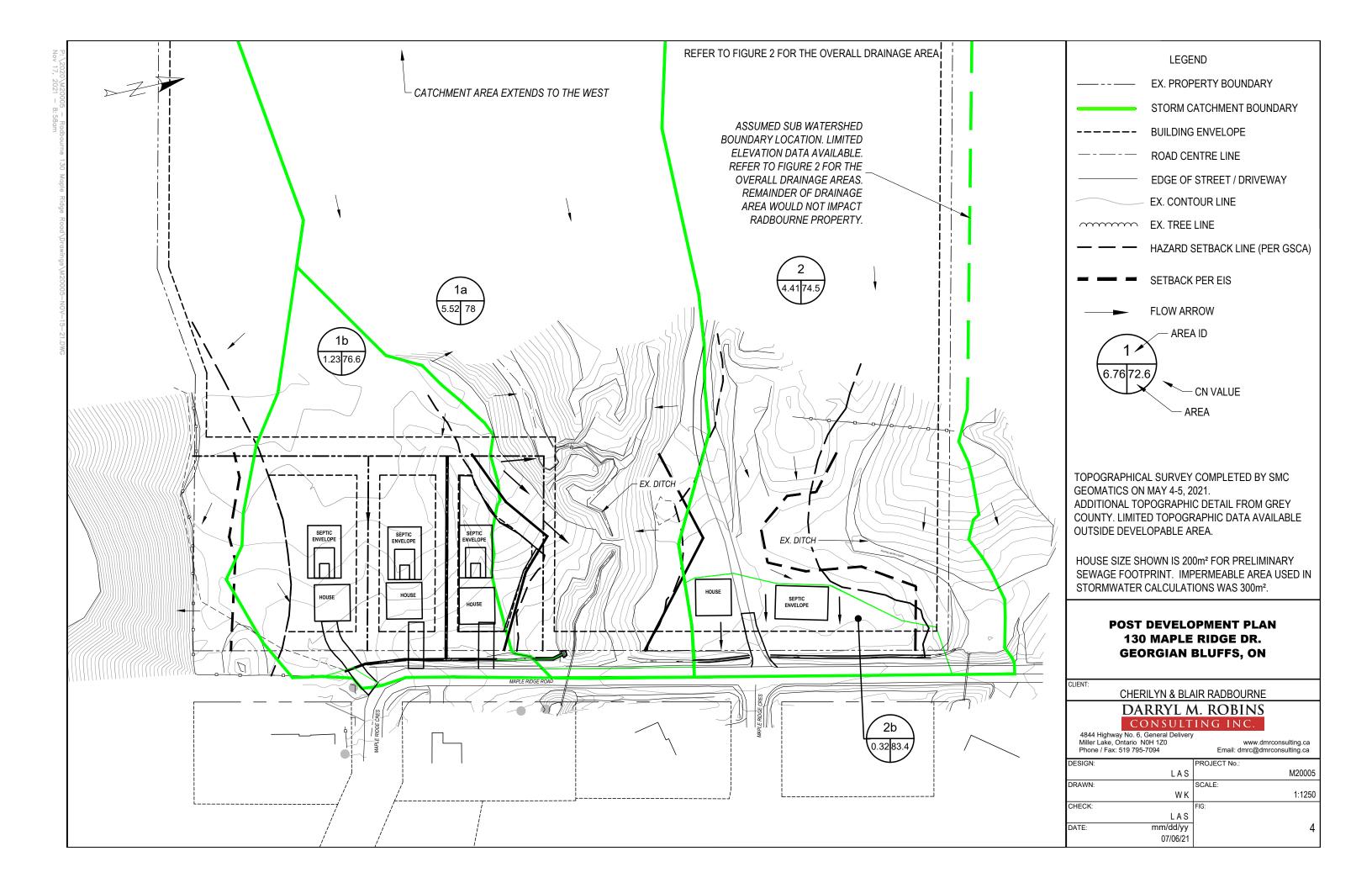


Table 3.3.3: Post-development Peak Flow

Aroa	Storm Event (Design Flow - L/s)									
Area	2	5	10	25	100					
1	40.36	108.44	189.78	282.77	440.62					
2	14.20	50.90	85.36	137.05	225.82					
System	52.89	157.61	274.71	418.9	665.94					

Overall, the post-development flows have been controlled to the pre-development levels using the grassed swales in the 2, 5, 25, 10 25 and the 100 Year design storm. Area 1's outlet post-development flow slightly exceeds the pre-development flow in the 100 Year design storm. Post development flow for area 1 is controlled for all the other storms noted above. There are some limitations to the controls due to the topography of the site. The outlet in the storm model was the downstream size of the culvert. The EIS identified that the central watercourse with fish habitat so we would like to minimize disturbance to this area. We considered increasing the base width of the grassed swale along lots 1 to 3 to 1m reduce the flow. But it appears that the flows through this property are significant, so it did not make an impact in further reducing the flows. Area 1 post development flow is 4.33 L/s higher than the pre-development flow which is not significant; it is less than 1% higher. Ultimately, we would consider the flows being controlled to pre-development levels.

In the 100 Year design storm the last section of the swale which discharges to the central watercourse is approximately 64% full. There is at least 0.5m clearance before the water would overtop the road at this location. The culvert crossing Maple Ridge Road which conveys the central watercourse is still surcharging in the 100 Year design storm. It was surcharging in the Pre-Development Condition and culverts are typically sized for the 25 Year design storm so this is acceptable. The last section of the swale fronting Lot 4 is approximately at 28% full in for the 100-year design storm. There is at least 0.4m clearance before the stormwater would overtop the road at this location in the 100-year storm.

4.0 SEDIMENT AND EROSION CONTROL

Precautions should be taken to ensure that erosion and sediment transport is controlled during the construction phases and until final landscaping covers have matured. The recommendations are as follows:

- 1. Temporary silt fence should be installed along the most down-gradient edge of any disturbed area on-site and maintained until the affected area has received final surfacing treatment and/or new vegetation has matured; and
- 2. Temporary straw bale check dams may be implemented where determined necessary during construction.

The measures above should be maintained until the proposed site surfacing is completed and vegetation has matured as applicable.

7.0 MAINTENANCE

The proposed grassed swale must be maintained by the Township. The Township should inspect the grass swale each spring and fall and after any major storm events. Accumulated sediment in the swale should be removed.

8.0 CONCLUSIONS

Based on the findings of this report, the following conclusions are made:

1. Stormwater management measures to address runoff quantity and quality concerns can be addressed successfully for the site. However, depending on the final building options (size and location of the dwelling) other design options could be considered.

9.0 RECOMMENDATIONS

It is recommended that the approval authorities support this development.

10.0 REFERENCES

The following documents were referenced in the preparation of the drainage study:

- MOE's Stormwater Management Planning & Design Manual (2003); and
- Soils of Grey County North Sheet Ontario (Soil Survey Report No. 17), Agriculture Canada, 1983.

Respectfully submitted by,

DARRYL M. ROBINS CONSULTING INC.

Lava Disa

Laura Swanson, P. Eng.

Vice-President

Civil-Environmental Engineer

Appendix A

Peto MacCallum Ltd's Test Pit Logs



LOG OF TEST PIT NO. 1 1 of 1 17T 504039E 4940961N PML REF. 21CX007 PROJECT Nitrate Study **ENGINEER** LOCATION 130 Maple Ridge Road BORING DATE July 28, 2021 GW EXCAVATION METHOD Rubber Track Excavator TECHNICIAN NT SHEAR STRENGTH (kPa) SAMPLES SOIL PROFILE **ELEVATION SCALE** +FIELD VANE △TORVANE ○ QU PLASTIC MATURAL MOISTURE LIMIT CONTENT LIQUID LIMIT WEIGHT GROUND WATER **OBSERVATIONS** STRAT PLOT VALUES NUMBER 100 50 150 200 DEPTH AND REMARKS DESCRIPTION ELEV LIND DYNAMIC CONE PENETRATION × STANDARD PENETRATION TEST GRAIN SIZE DISTRIBUTION (%) GR SA SI&CL metres) WATER CONTENT (%) z 10 20 30 40 60 80 SURFACE ELEVATION 204.30 kN/m 0.0 TOPSOIL: Dark brown, silty sand/sandy silt, trace organics, moist 204 0.70 203.60 SANDY SILT: Loose, brown to light brown, sandy silt, trace organics, trace gravel, trace clay, clay pockets, very 1.0 203 19 mm slotted pipe 2.0 GS 3.0 201.3 TEST PIT TERMINATED AT 3.0 m Upon completion of excavating: No sloughing Water at 2.9 m Water Level Readings: Depth(m) Elev. Date 2021-08-11 1.4 202.9 4.0 5.0 **NOTES**

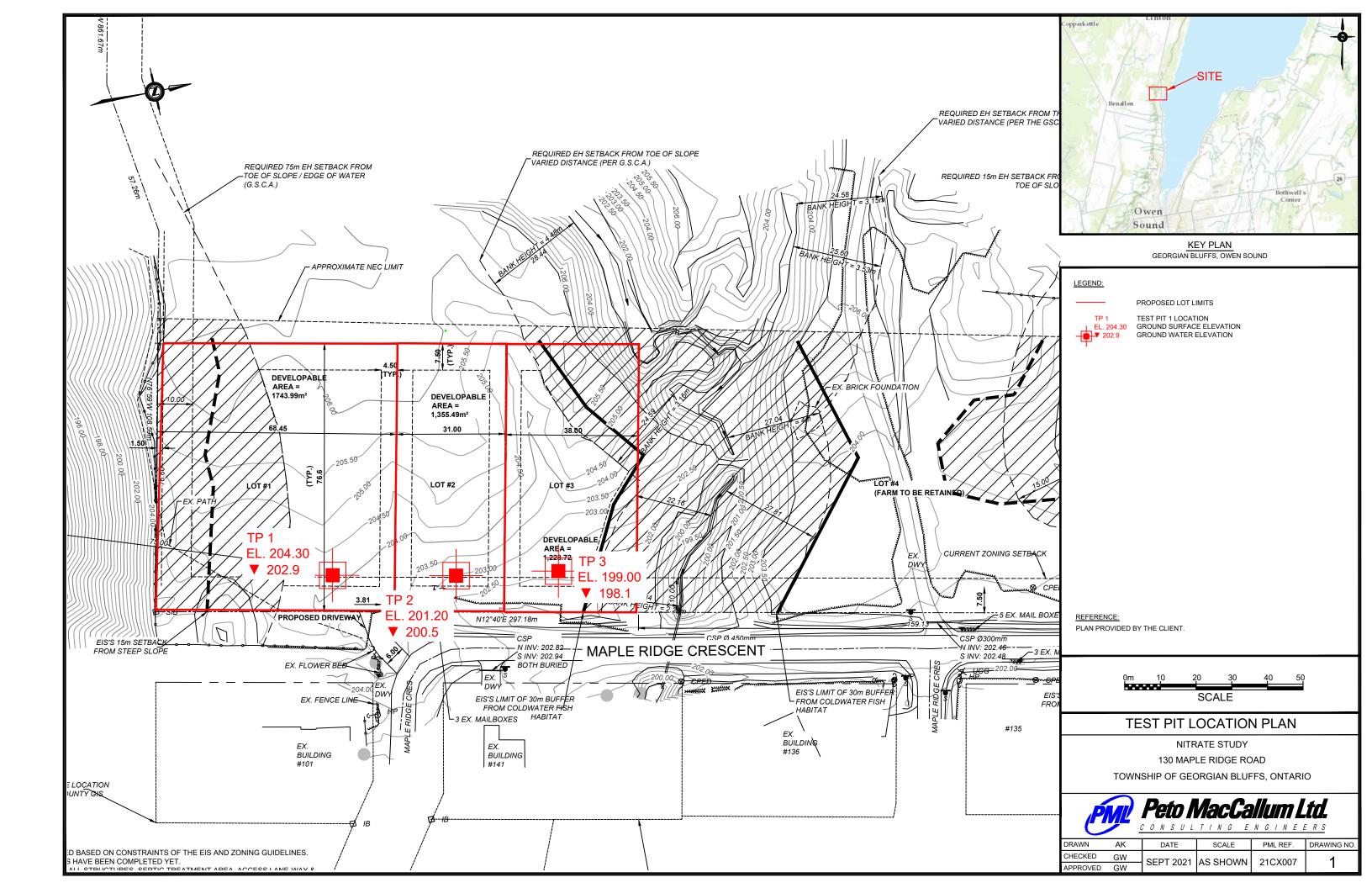


LOG OF TEST PIT NO. 2 1 of 1 17T 504027E 4941003N PML REF. PROJECT Nitrate Study 21CX007 BORING DATE July 28, 2021 LOCATION 130 Maple Ridge Road **ENGINEER** GW EXCAVATION METHOD Rubber Track Excavator TECHNICIAN NT SHEAR STRENGTH (kPa) SOIL PROFILE SAMPLES +FIELD VANE △TORVANE ○ Qu PLASTIC NATURAL MOISTURE A POCKET PENETROMETER O Q LIMIT CONTENT 50 100 150 200 W W W LIQUID LIMIT WEIGHT **GROUND WATER** STRAT PLOT **OBSERVATIONS** VALUES W DEPTH AND REMARKS DESCRIPTION ELEV LINO DYNAMIC CONE PENETRATION X STANDARD PENETRATION TEST GRAIN SIZE DISTRIBUTION (%) GR SA SI&CL metres) WATER CONTENT (%) 10 20 30 40 40 60 SURFACE ELEVATION 201.20 80 kN/m 0.0 TOPSOIL: Dark brown, silty sand/sandy silt, trace organics, moist 201.00 SANDY SILT: Loose, brown, sandy silt, trace clay, trace gravel, very moist to wet 1.0 200 19 mm slotted pipe 2.0 199 GS 1 SILT: Loose, grey, silt, trace sand, trace clay, very moist to wet 3.0 198.2 TEST PIT TERMINATED AT 3.0 m Upon completion of excavating: No sloughing Water at 2.9 m Water Level Readings: Date Depth(m) Elev. 2021-08-11 0.7 200.5 4.0 5.0 NOTES

PML - BH/TP LOG GEO/ENV WITH MWS 21CX007 TEST PIT LOGS 2021-07-30.GPJ ON_MOT.GDT 2021-09-13 3:27:51 PM



LOG OF TEST PIT NO. 3 1 of 1 17T 504030E 4941032N PML REF. 21CX007 PROJECT Nitrate Study BORING DATE July 28, 2021 **ENGINEER** GW LOCATION 130 Maple Ridge Road **EXCAVATION METHOD** Rubber Track Excavator TECHNICIAN NT SHEAR STRENGTH (kPa) SAMPLES SOIL PROFILE +FIELD VANE ATORVANE O QUI PLASTIC MOISTURE LIMIT CONTENT LIQUID LIMIT WEIGHT GROUND WATER **OBSERVATIONS** STRAT PLOT NUMBER VALUES ELEVATION DEPTH ELEV 100 150 200 AND REMARKS DESCRIPTION LIND DYNAMIC CONE PENETRATION X STANDARD PENETRATION TEST GRAIN SIZE DISTRIBUTION (%) GR SA SI&CL (metres WATER CONTENT (%) ż 40 60 10 20 30 SURFACE ELEVATION 199.00 kN/m3 0.0 TOPSOIL: Dark brown, silty sand/sandy silt, trace organics, moist 0.40 198.60 SILT: Loose, brown, silt, some sand, trace clay, trace gravel, very moist 198 1.0 19 mm slotted pipe 197 2.0 1 GS 3.0 Upon completion of excavating: No sloughing 196.0 TEST PIT TERMINATED AT 3.0 m Water at 2.9 m Water Level Readings: Depth(m) Elev. Date 2021-08-11 0.9 198.1 4.0 5.0 **NOTES**



Appendix B

GSCA Correspondence



237897 Inglis Falls Road, R.R.#4, Owen Sound, ON N4K 5N6
Telephone: 519.376.3076 Fax: 519.371.0437
www.greysauble.on.ca

November 22, 2019

Cherilyn Anne Radbourne & Blair Edward Radbourne 558 Punkinseed Lane Kemble, ON N0H 1S0

Dear Cherilyn and Blair,

RE: Potential Severances for Residential Development

Part of Lot 17, Concession 2

130 Maple Ridge Road; Roll No. 42-03-580-003-033-00 Township of Georgian Bluffs, former Sarawak Township

Our File: P19467

Grey Sauble Conservation Authority (GSCA) has reviewed the subject property at your request to consider potential severance applications to create three new lots for residential development purposes and leaving one retained lot. GSCA has conducted this preliminary review in accordance with our mandate and policies for natural hazards, for natural heritage concerns under the Planning Act, and relative to our policies for the implementation of Ontario Regulation 151/06. We offer the following comments.

Site Description

The property is located on the west side of Maple Ridge Road, approximately 180 metres south of Grey Road 1, in the Township of Georgian Bluffs, former Sarawak Township. The property is approximately 31 hectares (77 acres) in area and is currently vacant. Historically the property featured structures, which were recently demolished. Portions of the property are utilized for agricultural purposes with naturalized areas featuring woodlands, watercourses and valleys. Given the size of the property and the conceptual proposal, GSCA's site inspection was limited to the eastern portion of the property within the area subject to municipal zoning. We note, a large portion of the property falls within Niagara Escarpment Development Control. The eastern portion of the property features two minor valley systems that convey watercourses easterly to their outlet at Georgian Bay. A large valley feature associated with Indian Creek is adjacent to the south of the property. The south east woodland is comprised largely of upland deciduous trees with an understory consisting of ostrich fern and sensitive fern. The north eastern woodland features some mixed tree species and is more evident of historical site disturbance as a result of previously existing development and anthropogenic activities on the property.

GSCA Regulations

Portions of the property are regulated under Ontario Regulation 151/06: Regulation of Development, Interference with Wetlands & Alteration to Shorelines & Watercourses. The regulated areas within the eastern portion of the property include the meander belt and valley lands of two unnamed watercourses, and the valley of Indian Creek.



1 of 4

Watershed Municipalities

Under this regulation a permit is required from this office prior to the construction, reconstruction, erection or placing of a building or structure of any kind; any change to a building or structure that would have the effect of altering the use or potential use of the building or structures, increasing the size of the building or structure, or increasing the number of dwelling units in the building or structure; site grading; or, the temporary or permanent placing, dumping or removal of any material originating on the site or elsewhere, if occurring within the regulated area. Also, a permit is required for interference with a wetland, and/or the straightening, changing, diverting or in any way interfering with an existing channel of a river, lake, creek stream or watercourse.

The regulated areas are shown on the attached map. Please note, if there is a conflict between the description of the areas regulated and the map, the description of regulated areas prevails.

Provincial Policy Statement

3.1 Natural Hazards

Flood and erosion hazards on the eastern portion of the property are associated with the minor valley systems and the valley of Indian Creek.

- 3.1.1. Development shall generally be directed to areas outside of:
 - b) hazardous lands adjacent to river, stream and small inland lake systems which are impacted by flooding hazards and/or erosion hazards.

The extent of the slope hazard associated with the steep valley wall of Indian Creek includes an assumed stable slope line measured at 3 horizontal units to 1 vertical unit (3:1) applied from the toe of the slope to a point of intersection with the earth's surface. Where a watercourse is located at the toe of the slope an additional 15 metres is applied for toe erosion. Based on the newly acquired 0.5 metre LiDAR contours and additional review in 3D stereo pairs and 5 metre contours, we estimate the height of the valley slope to be 20 metres. As such, the 3:1 plus 15 metre toe erosion allowance is considered to be 75 metres applied from the toe of the slope. Any reduction in this allowance is to be supported with by a geotechnical study completed by a qualified professional engineer with the appropriate consideration for a factor of safety.

The height of the valley associated with the central watercourse feature was found to be in the range of 3.5 – 4 metres depending on the location with evidence of minor erosion occurring. The hazard area was mapped in accordance with the same criteria noted in the previous paragraph (3:1 plus 15 metre). Further assessment by an engineer is required to consider any reductions in the hazard allowance.

The northerly watercourse does not feature a well-defined valley within the area subject to municipal zoning. As such, a minimum 15 metre allowance was applied from the watercourse for flood and erosion concerns.

The attached map shows the revised hazard mapping for the subject property. These areas are recommended to be included in the Township Comprehensive Zoning By-law as Environmental Protection.

2.1 Natural Heritage

Natural heritage features identified on the subject property include significant woodland as mapped in the County of Grey Official Plan, potential for significant wildlife habitat, indirect fish habitat associated with the unnamed watercourses, potential for habitat of endangered species and threatened species,

adjacent lands to the natural heritage features, including fish habitat associated with Indian Creek. The following policies are relevant:

- 2.1.5 Development and site alteration shall not be permitted in:
 - b) significant woodlands in Ecoregions 6E and 7E (excluding islands in Lake Huron and the St. Marys River);
 - d) significant wildlife habitat;
- 2.1.6 Development and site alteration shall not be permitted in fish habitat except in accordance with provincial and federal requirements.
- 2.1.7 Development and site alteration shall not be permitted in habitat of endangered species and threatened species, except in accordance with provincial and federal requirements.
- 2.1.8 Development and site alteration shall not be permitted on adjacent lands to the natural heritage features and areas identified in policies 2.1.4, 2.1.5, and 2.1.6 unless the ecological function of the adjacent lands has been evaluated and it has been demonstrated that there will be no negative impacts on the natural features or on their ecological functions.

Given the above policies and GSCA's review of the property, a satisfactory Environmental Impact Study (EIS) is warranted as part of any severance application submitted for the property. We recommend your chosen consultant contact our office to establish a terms of reference prior to commencing any study work.

Stormwater Management

Surface drainage within the eastern portion of the property appears to be largely directed to the smaller valley features and watercourses, which flow easterly towards their outlet in Georgian Bay. As the area being considered for the severances features woodlands and pervious surfaces, the proposed severances have the potential to increase peak flows leaving the site, which may impact downstream areas. We recommend any consent proposal include a stormwater management report completed by a qualified engineer to address stormwater quantity and quality concerns. Part of this report is to include a topography survey outlining the proposed lots with limits of proposed development.

Additional Comments & Recommendations

From GSCA's perspective, the following studies/technical information is to be competed for consideration of the formal consent applications:

- 1. Environmental Impact Study completed by a qualified professional. We recommend a terms of reference be established prior to commencing any study work.
- 2. Stormwater Management Report completed by a qualified engineer. The report is to demonstrate how the proposal addresses stormwater quantity and quality concerns.
- 3. Topographic plan for the proposed lots and development areas

We recommend consulting with the Township of Georgian Bluffs, Grey County, and the Niagara Escarpment Commission to determine any other requirements for the proposal.

November 22, 2019 Our File No. P19467

We note, the delineation of the regulated and hazard areas on the attached map may be subject to change pending the outcome of the topographic survey. GSCA reserves the right to request more information and/or clarification of the proposal as more details are presented. These comments are considered preliminary at this time and for pre-consultation purposes only.

If any questions should arise, please contact the undersigned.

Sincerely

Mac Plewes

Watershed Planner

Enclosure

cc via email: Mr. Dwight Burley, GSCA Director, Township of Georgian Bluffs

Ms. Sue Carleton, GSCA Director, Township of Georgian Bluffs

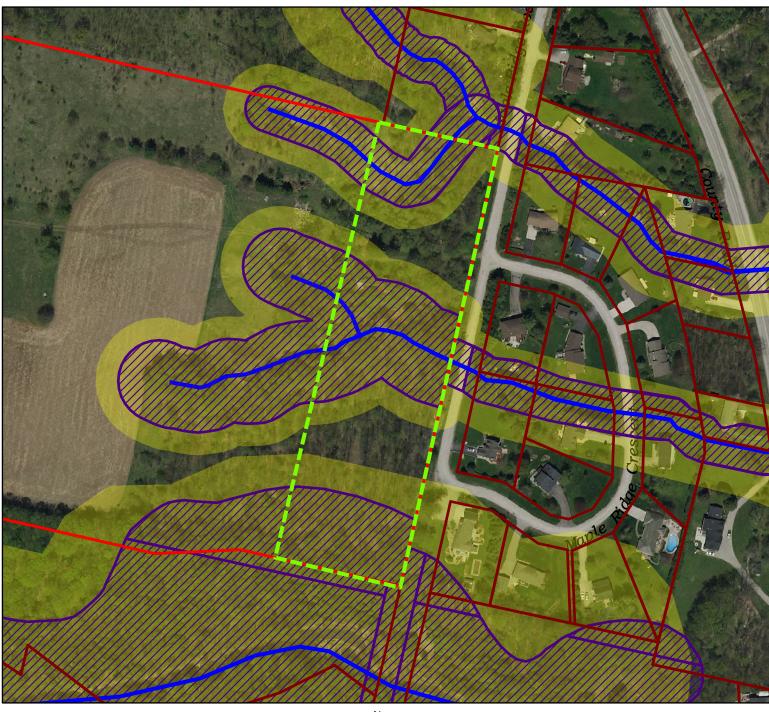
Mr. Rick Winters, CAO/Director of Operations, Township of Georgian Bluffs Mr. Brandon Henderson, Senior Planner, Niagara Escarpment Commission

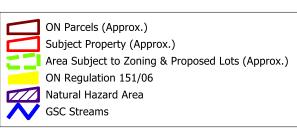
Planning & Building Departments, Township of Georgian Bluffs

Planning Department, County of Grey

Mr. Dennis Radbourne

GSCA: Regulation of Development, Interference with Wetlands and Alterations to Shorelines and Watercourses (Ontario Regulation 151/06)







Scale = 1:2500

0 50 m

GSCA Map 130 Maple Ridge Road Roll No. 42-03-580-003-033-00 Township of Georgian Bluffs (Sarawak) Our File: P19467

Friday, November 22, 2019

the included mapping has been compiled from various sources and is for information purposes only. Grey Stable Conservation is not responsible for, and cannot guarantee, the accuracy of all electromation contained within the map. Regulation lines were created by Grey Stable Conservation (GSC) using I neere consum interpolated from the Provincial (10 meter) Digital Electromation contained within the map. Regulation lines were created by Grey Stable Conservation (GSC) using I neere consum interpolated from the Provincial (10 meter) Digital Electromation contained within the map. Regulation lines were created by Grey Stable Conservation (GSC) using I neere consuming the contract of the provincial (10 meter) Digital Electromation contained within the map. Regulation lines were created by Grey Stable Conservation (GSC) using I neere consuming the contract of the contract

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This mapping contains products of the South Western Ontario Orthophotography Project (SWOOP). These images were taken in 2015 at 20cm



Appendix C

Stormwater Management Calculations & Modelling



Active coordinate

44° 36' 15" N, 80° 57' 14" W (44.604167,-80.954167)

Retrieved: Mon, 28 Jun 2021 17:25:30 GMT



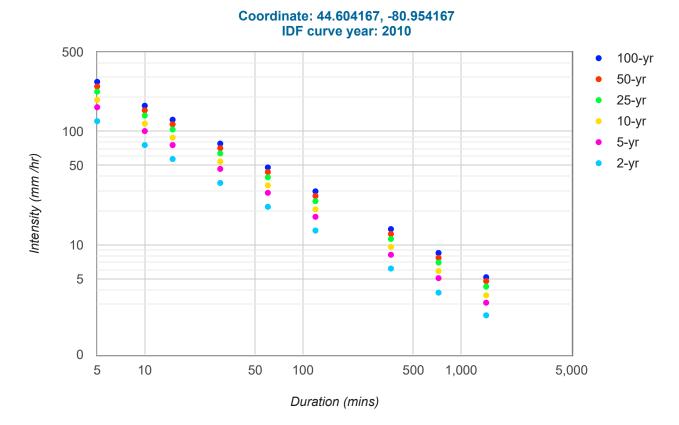
Location summary

These are the locations in the selection.

IDF Curve: 44° 36' 15" N, 80° 57' 14" W (44.604167,-80.954167)

Results

An IDF curve was found.



Coefficient summary

IDF Curve: 44° 36' 15" N, 80° 57' 14" W (44.604167,-80.954167)

Retrieved: Mon, 28 Jun 2021 17:25:30 GMT

Data year: 2010 IDF curve year: 2010

Return period	2-yr	5-yr	10-yr	25-yr	50-yr	100-yr
Α	21.7	28.8	33.5	39.5	43.9	48.2
В	-0.699	-0.699	-0.699	-0.699	-0.699	-0.699

Statistics

Rainfall intensity (mm hr⁻¹)

Duration	5-min	10-min	15-min	30-min	1-hr	2-hr	6-hr	12-hr	24-hr
2-yr	123.3	75.9	57.2	35.2	21.7	13.4	6.2	3.8	2.4
5-yr	163.6	100.8	75.9	46.8	28.8	17.7	8.2	5.1	3.1
10-yr	190.3	117.2	88.3	54.4	33.5	20.6	9.6	5.9	3.6
25-yr	224.4	138.2	104.1	64.1	39.5	24.3	11.3	7.0	4.3
50-yr	249.4	153.6	115.7	71.3	43.9	27.0	12.5	7.7	4.8
100-yr	273.8	168.6	127.0	78.2	48.2	29.7	13.8	8.5	5.2

Rainfall depth (mm)

Duration	5-min	10-min	15-min	30-min	1-hr	2-hr	6-hr	12-hr	24-hr
2-yr	10.3	12.7	14.3	17.6	21.7	26.7	37.2	45.8	56.5
5-yr	13.6	16.8	19.0	23.4	28.8	35.5	49.4	60.8	75.0
10-yr	15.9	19.5	22.1	27.2	33.5	41.3	57.4	70.8	87.2
25-yr	18.7	23.0	26.0	32.1	39.5	48.7	67.7	83.5	102.8
50-yr	20.8	25.6	28.9	35.6	43.9	54.1	75.3	92.7	114.3
100-yr	22.8	28.1	31.8	39.1	48.2	59.4	82.7	101.8	125.5

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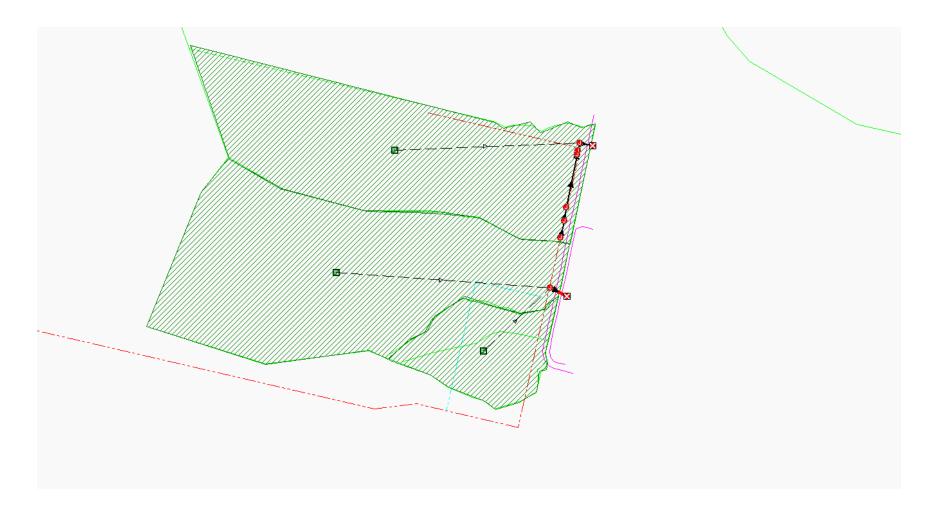
Last Modified: September 2016

TC Calculations

SubCatchment	Area (ha)	Slope (%)	Length (m)	Tc (min)	Tc(hr)
Area 1	6.76	2.83	428.0	16.36	0.27
1a	5.53	2.83	428.0	16.70	0.28
1b	1.24	2.80	178.3	8.09	0.13
2	4.16	1.51	472.6	21.50	0.36
2b	0.32	3.52	33.0	1.64	0.03

Calculated using the Bransby William Formula

Pre Development



Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-PRE-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ****** Number of rain gages 1 Number of subbasins 3 Number of nodes 9 Number of links 7 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 2 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares _____

 Sub-01a
 5.53
 484.00

 Sub-01b
 1.24
 484.00

 Sub-02
 4.48
 484.00

 ******* Node Summary Element Invert Maximum Ponded External Type Elevation Elev. Area Inflow m m m^2 Node _____
 Jun-01
 JUNCTION
 198.90
 201.00
 0.00

 Jun-02
 JUNCTION
 199.84
 201.60
 0.00

 Jun-03
 JUNCTION
 201.60
 202.00
 0.00

 Jun-04
 JUNCTION
 202.46
 204.00
 0.00

 Jun-05
 JUNCTION
 202.48
 204.00
 0.00

 Jun-06
 JUNCTION
 202.72
 203.05
 0.00

Jun-07 Out-01 Out-02	JUNCTION OUTFALL OUTFALL		201.87 198.56 199.65	202.10 199.01 200.15	0.00 0.00 0.00		

Link ID	From Node	To Node		Element Type	Length m	. ે	Manning's Roughness
Link-01 Link-02 Link-03 Link-04 Link-05 Link-06 Link-07	Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07	Out-01 Out-02 Jun-02 Jun-07 Jun-04 Jun-05 Jun-03		CONDUIT CONDUIT CHANNEL CHANNEL CONDUIT CHANNEL CHANNEL	19.0 12.7 10.0 63.2 8.2 21.0	1.7867 1.4961 17.6000 0.9328 0.2439 1.1407	0.0150 0.0150 0.0320 0.0320 0.0150 0.0320 0.0320
**************************************	Summary						
Link	Shape	Depth	/	Width	No. of	Cross	Full Flow
Design ID		Diameter	r		Barrels	Sectional	Hydraulic
Flow						Area	Radius
Capacity		T	n	m		m²	m
LPS							
Link-01 330.30	CIRCULAR	0.4	5	0.45	1	0.16	0.11
Link-02 400.29	CIRCULAR	0.50	0	0.50	1	0.20	0.12
Link-03 926.98	TRAPEZOIDAL	0.20)	1.95	1	0.27	0.13
Link-04 243.76	TRAPEZOIDAL	0.20	ס	2.90	1	0.34	0.12
Link-05 41.39	CIRCULAR	0.30)	0.30	1	0.07	0.07
Link-06	TRAPEZOIDAL	0.20)	2.50	1	0.30	0.12
241.07 Link-07 201.32	TRAPEZOIDAL	0.10	0	2.20	1	0.16	0.07
Runoff Quanti	************ ty Continuity ******	Volume hectare-r	n	Depth mm			
Total Precipitation Surface Runoff Continuity Error (%)		0.410 0.141 -0.003	1	37.024 12.547			
********	*****	Volume	e	Volume			
Flow Routing	Continuity	hectare-r		Mliters			
External Outf Initial Store Final Stored	ow	0.000 0.049 0.000 0.000	9 0 0	0.000 0.495 0.000 0.001			

******	*****	*****	****				
Composite Curve							
Subbasin Sub-01a							
Soil/Surface Des					Area (ha)	Soil Group	CN
-					5.53	_	78.00
Composite Area &	Weighted CN				5.53		78.00
Subbasin Sub-01b							
					Area	Soil	
Soil/Surface Des	cription					Group	CN
-					1.24		73.40
Composite Area &	Weighted CN				1.24		73.40
Subbasin Sub-02							
					_	G ! 1	
Soil/Surface Des	_				Area (ha)	Soil Group	CN
					4.48	-	75.30
- Composite Area &	Weighted CN				0.00 4.48	_	0.00 75.30
_					1.10		73.30
****************** Subbasin Runoff							

Subbasin	Total	Total	Peak	Weighted Curve	_	Time of	
ID	mm	mm	LPS	Number	days	hh:mm:ss	
 Sub-01a	37.20			78.000			
Sub-01b	37.20	3.17	3.68	73.400	0	00:08:05	
Sub-02	37.20	4.06	17.56	75.300	0	00:21:20	

Node Depth Summary

Node ID	Average Depth Attained	Maximum Depth Attained	Maximum HGL Attained		of Max urrence	Total Flooded Volume	Total Time Flooded	Retention Time
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.11	0.20	199.10	0	02:45	0	0	0:00:00
Jun-02	0.10	0.13	199.97	0	02:52	0	0	0:00:00
Jun-03	0.00	0.00	201.60	0	00:00	0	0	0:00:00
Jun-04	0.00	0.00	202.46	0	00:00	0	0	0:00:00
Jun-05	0.00	0.00	202.48	0	00:00	0	0	0:00:00
Jun-06	0.00	0.00	202.72	0	00:00	0	0	0:00:00
Jun-07	0.00	0.00	201.87	0	00:00	0	0	0:00:00
Out-01	0.07	0.11	198.67	0	02:45	0	0	0:00:00
Out-02	0.05	0.08	199.73	0	02:52	0	0	0:00:00

Node ID	Element Type	Maximum Lateral Inflow	Peak Inflow	Time of Peak Inflow Occurrence		Maximum Flooding Overflow	Time of Peak Flooding Occurrence
		LPS	LPS	days	hh:mm	LPS	days hh:mm
Jun-01	JUNCTION	44.03	44.03	0	02:45	0.00	
Jun-02	JUNCTION	17.30	17.30	0	02:54	0.00	
Jun-03	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-04	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-05	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-06	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00	
Out-01	OUTFALL	0.00	46.79	0	02:45	0.00	
Out-02	OUTFALL	0.00	22.19	0	02:52	0.00	

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	85.24	25.95	46.79
Out-02	84.34	13.64	22.19
System	84.79	39.58	64.99

Link ID Ratio of		Element tal Reported	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
		Type	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
Maximum	Ti	me Condition	0ccu	rrence	Attained		Analysis	Capacity	/Design
Flow Surcharged								, 5	
			days	hh:mm	m/sec		LPS	LPS	Flow
Depth min	utes								
Link-01		CONDUIT	0	02:45	1.05	1.00	46.79	330.30	0.14
0.35	0	Calculated							
Link-02		CONDUIT	0	02:52	0.83	1.00	22.19	400.29	0.06
0.21	0	Calculated	•	00.00	0.00	1 00	0.00	006.00	0.00
Link-03 0.32	0	CHANNEL Calculated	0	00:00	0.00	1.00	0.00	926.98	0.00
0.32 Link-04	U	CHANNEL	0	00:00	0.00	1.00	0.00	243.76	0.00
0.00	0	Calculated	U	00.00	0.00	1.00	0.00	243.70	0.00
Link-05	Ū	CONDUIT	0	00:00	0.00	1.00	0.00	41.39	0.00
0.00	0	Calculated							
Link-06		CHANNEL	0	00:00	0.00	1.00	0.00	241.07	0.00

2 Year Pre-Development Radbourne Severance

********* Highest Flow Instability Indexes

Link Link-02 (22) Link Link-01 (4)

Analysis began on: Wed Nov 17 10:18:25 2021 Analysis ended on: Wed Nov 17 10:18:26 2021 Total elapsed time: 00:00:01

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-PRE-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ****** Number of rain gages 1 Number of subbasins 3 Number of nodes 9 Number of links 7 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 5 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares _____

 Sub-01a
 5.53
 484.00

 Sub-01b
 1.24
 484.00

 Sub-02
 4.48
 484.00

 ******* Node Summary Element Invert Maximum Ponded External Type Elevation Elev. Area Inflow m m \mathfrak{m}^2 Node _____
 Jun-01
 JUNCTION
 198.90
 201.00
 0.00

 Jun-02
 JUNCTION
 199.84
 201.60
 0.00

 Jun-03
 JUNCTION
 201.60
 202.00
 0.00

 Jun-04
 JUNCTION
 202.46
 204.00
 0.00

 Jun-05
 JUNCTION
 202.48
 204.00
 0.00

 Jun-06
 JUNCTION
 202.72
 203.05
 0.00

Jun-07 Out-01 Out-02	JUNCTION OUTFALL OUTFALL		201.87 198.56 199.65	202.10 199.01 200.15	0.00 0.00 0.00		

Link ID	From Node	To Node		Element Type	Length m	Slope %	Manning's Roughness
Link-01 Link-02 Link-03 Link-04 Link-05 Link-06 Link-07	Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07	Out-01 Out-02 Jun-02 Jun-07 Jun-04 Jun-05 Jun-03		CONDUIT CONDUIT CHANNEL CHANNEL CONDUIT CHANNEL CHANNEL	19.0 12.7 10.0 63.2 8.2 21.0 5.0	1.7867 1.4961 17.6000 0.9328 0.2439 1.1407 5.4000	0.0150 0.0150 0.0320 0.0320 0.0150 0.0320 0.0320

Cross Section	*****						
Link Design	Shape	Depth/		Width	No. of	Cross	Full Flow
ID Flow		Diameter	•		Barrels	Sectional	Hydraulic
						Area	Radius
Capacity		m	ı	m		m²	m
LPS							
Link-01 330.30	CIRCULAR	0.45	i	0.45	1	0.16	0.11
Link-02 400.29	CIRCULAR	0.50)	0.50	1	0.20	0.12
Link-03	TRAPEZOIDAL	0.20)	1.95	1	0.27	0.13
926.98 Link-04	TRAPEZOIDAL	0.20)	2.90	1	0.34	0.12
243.76 Link-05	CIRCULAR	0.30	1	0.30	1	0.07	0.07
41.39 Link-06	TRAPEZOIDAL	0.20)	2.50	1	0.30	0.12
241.07 Link-07 201.32	TRAPEZOIDAL	0.10	ı	2.20	1	0.16	0.07
**************************************	ty Continuity	Volume	ı	Depth mm			
Total Precipit Surface Runof: Continuity Err	f	0.553 0.310 -0.004)	49.167 27.599			
**************************************	Continuity	Volume	ı	Volume Mliters			
External Inflo External Outf Initial Stored Final Stored V Continuity Err	lowd Volume Volume	0.000 0.109 0.000 0.000 -0.000) 	0.000 1.087 0.000 0.002			

Composite Curve Number Computations Report Subbasin Sub-01a Area Soil (ha) Group CN Soil/Surface Description 5.53 - 78.00 5.53 78.00 Composite Area & Weighted CN Subbasin Sub-01b Area Soil Area Soll (ha) Group CN Soil/Surface Description - 73.40 73.40 1.24 Composite Area & Weighted CN 1.24 Subbasin Sub-02 ______ Soil Area Soil (ha) Group CN Soil/Surface Description ______ - 75.30 - 0.00 75.30 4.48 0.00 Composite Area & Weighted CN 4.48 Subbasin Runoff Summary Subbasin Total Total Peak Weighted Time of ID Precip Runoff Runoff Curve Concentration mm mm LPS Number days hh:mm:ss
 Sub-01a
 49.40
 11.53
 111.00
 78.000
 0 00:16:42

 Sub-01b
 49.40
 7.80
 16.42
 73.400
 0 00:08:05

 Sub-02
 49.40
 9.25
 59.18
 75.300
 0 00:21:20

Node Depth Summary

Node ID	Average Depth Attained	Maximum Depth Attained	Maximum HGL Attained		of Max urrence	Total Flooded Volume	Total Time Flooded	Retention Time
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.20	0.39	199.29	0	02:39	0	0	0:00:00
Jun-02	0.12	0.19	200.03	0	02:45	0	0	0:00:00
Jun-03	0.00	0.00	201.60	0	00:00	0	0	0:00:00
Jun-04	0.00	0.00	202.46	0	00:00	0	0	0:00:00
Jun-05	0.00	0.00	202.48	0	00:00	0	0	0:00:00
Jun-06	0.00	0.00	202.72	0	00:00	0	0	0:00:00
Jun-07	0.00	0.00	201.87	0	00:00	0	0	0:00:00
Out-01	0.11	0.19	198.75	0	02:39	0	0	0:00:00
Out-02	0.08	0.13	199.78	0	02:45	0	0	0:00:00

Node ID	Element Type	Maximum Lateral	Peak Inflow	_	ime of Inflow	Maximum Flooding	Time of Peak Flooding
		Inflow		Occu	rrence	Overflow	Occurrence
		LPS	LPS	days	hh:mm	LPS	days hh:mm
Jun-01	JUNCTION	125.76	125.76	0	02:40	0.00	
Jun-02	JUNCTION	59.27	59.27	0	02:45	0.00	
Jun-03	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-04	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-05	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-06	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00	
Out-01	OUTFALL	0.00	125.98	0	02:39	0.00	
Out-02	OUTFALL	0.00	59.11	0	02:45	0.00	

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	90.23	54.54	125.98
Out-02	89.70	30.51	59.11
System	89.96	85.05	179.36

Link ID Ratio of		Element tal Reported	T	ime of	Maximum	Length	Peak Flow	Design	Ratio of
		Type	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
Maximum	Ti	me Condition	0ccu:	rrence	Attained		Analysis	Capacity	/Design
Flow Surchar	ged		days	hh:mm	m/sec		LPS	LPS	Flow
Depth min	utes								
Link-01	0	CONDUIT	0	02:39	1.15	1.00	125.98	330.30	0.38
0.65 Link-02	Ü	Calculated CONDUIT	0	02:45	1.11	1.00	59.11	400.29	0.15
0.32 Link-03	0	Calculated CHANNEL	0	00:00	0.00	1.00	0.00	926.98	0.00
0.46 Link-04	0	Calculated CHANNEL	0	00:00	0.00	1.00	0.00	243.76	0.00
0.00 Link-05	0	Calculated CONDUIT	0	00:00	0.00	1.00	0.00	41.39	0.00
0.00	0	Calculated	-						
Link-06		CHANNEL	0	00:00	0.00	1.00	0.00	241.07	0.00

5 Year Pre-Development Radbourne Severance

********* Highest Flow Instability Indexes

Link Link-02 (58) Link Link-01 (14)

Analysis began on: Wed Nov 17 10:20:40 2021 Analysis ended on: Wed Nov 17 10:20:41 2021 Total elapsed time: 00:00:01

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-PRE-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ****** Number of rain gages 1 Number of subbasins 3 Number of nodes 9 Number of links 7 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min Rain Gage-01 10 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares _____

 Sub-01a
 5.53
 484.00

 Sub-01b
 1.24
 484.00

 Sub-02
 4.48
 484.00

 ******* Node Summary Element Invert Maximum Ponded External Type Elevation Elev. Area Inflow m m \mathfrak{m}^2 Node _____
 Jun-01
 JUNCTION
 198.90
 201.00
 0.00

 Jun-02
 JUNCTION
 199.84
 201.60
 0.00

 Jun-03
 JUNCTION
 201.60
 202.00
 0.00

 Jun-04
 JUNCTION
 202.46
 204.00
 0.00

 Jun-05
 JUNCTION
 202.48
 204.00
 0.00

 Jun-06
 JUNCTION
 202.72
 203.05
 0.00

Jun-07 Out-01 Out-02	JUNCTION OUTFALL OUTFALL		201.87 198.56 199.65	202.10 199.01 200.15	0.00 0.00 0.00		

Link ID	From Node	To Node		Element Type	Length m	Slope %	Manning's Roughness
Link-01 Link-02 Link-03 Link-04 Link-05 Link-06 Link-07	Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07	Out-01 Out-02 Jun-02 Jun-07 Jun-04 Jun-05 Jun-03		CONDUIT CONDUIT CHANNEL CHANNEL CONDUIT CHANNEL CHANNEL	19.0 12.7 10.0 63.2 8.2 21.0	1.7867 1.4961 17.6000 0.9328 0.2439 1.1407 5.4000	0.0150 0.0150 0.0320 0.0320 0.0320 0.0150 0.0320 0.0320

*********** Link	******** Shape	Depth	/	Width	No. of	Cross	Full Flow
Design ID	2334	Diamete			Barrels	Sectional	Hydraulic
Flow		Diameter	-		Dallelb		
Capacity						Area	Radius
LPS		г	n	m		m²	m
Link-01 330.30	CIRCULAR	0.45	5	0.45	1	0.16	0.11
Link-02 400.29	CIRCULAR	0.50)	0.50	1	0.20	0.12
Link-03 926.98	TRAPEZOIDAL	0.20)	1.95	1	0.27	0.13
Link-04 243.76	TRAPEZOIDAL	0.20)	2.90	1	0.34	0.12
Link-05	CIRCULAR	0.30)	0.30	1	0.07	0.07
41.39 Link-06	TRAPEZOIDAL	0.20)	2.50	1	0.30	0.12
241.07 Link-07 201.32	TRAPEZOIDAL	0.10)	2.20	1	0.16	0.07
Runoff Quanti	************** ity Continuity *******	Volume hectare-r	n	Depth mm			
Total Precipi Surface Runof	itation Ef cror (%)	0.643 0.445 -0.004	3	57.129 39.531			
Flow Routing	************** Continuity ***********************************	Volume hectare-r	n	Volume Mliters			
External Infl External Outf Initial Store Final Stored	low	0.000 0.150 0.000 0.000) 5)	0.000 1.557 0.000 0.002			

**************************************	Number Comput	ations Re	port				
 Subbasin Sub-01a							
Soil/Surface Desc					Area (ha)	Soil Group	CN
- Composite Area &					5.53 5.53	-	
 Subbasin Sub-01b							
Soil/Surface Desc					Area (ha)	Soil Group	
- Composite Area &					1.24	-	73.40 73.40
Subbasin Sub-02							
Soil/Surface Desc	cription				Area (ha)	Soil Group	CN
Composite Area &	****** Summary				4.48 0.00 4.48	- - -	75.30 0.00 75.30
Subbasin ID	Total Precip mm	Total Runoff mm	Peak Runoff LPS	Weighted Curve Number	Conc days	Time of entration hh:mm:ss	
Sub-01a Sub-01b Sub-02	57.40	16.18	167.35	78.000 73.400 75.300	0	00:16:42 00:08:05 00:21:20	

Node Depth Summary

Node	Average	Maximum	Maximum	Time	of Max	Total	Total	Retention
ID	Depth	Depth	HGL	Occi	ırrence	Flooded	Time	Time
	Attained	Attained	Attained			Volume	Flooded	
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.25	0.54	199.44	0	02:39	0	0	0:00:00
Jun-02	0.15	0.29	200.13	0	02:45	0	0	0:00:00
Jun-03	0.00	0.00	201.60	0	00:00	0	0	0:00:00
Jun-04	0.00	0.00	202.46	0	00:00	0	0	0:00:00
Jun-05	0.00	0.00	202.48	0	00:00	0	0	0:00:00
Jun-06	0.00	0.00	202.72	0	00:00	0	0	0:00:00
Jun-07	0.00	0.00	201.87	0	00:00	0	0	0:00:00
Out-01	0.13	0.25	198.81	0	02:40	0	0	0:00:00
Out-02	0.10	0.17	199.82	0	02:45	0	0	0:00:00

Node	Element	Maximum	Peak	Т	ime of	Maximum	Time of	Peak
ID	Type	Lateral	Inflow	Peak	Inflow	Flooding	Flo	oding
		Inflow		0ccu	rrence	Overflow	Occur	rence
		LPS	LPS	days	hh:mm	LPS	days	hh:mm
Jun-01	JUNCTION	191.32	191.32	0	02:39	0.00		
Jun-02	JUNCTION	95.60	95.60	0	02:44	0.00		
Jun-03	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-04	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-05	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-06	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00		
Out-01	OUTFALL	0.00	191.27	0	02:40	0.00		
Out-02	OUTFALL	0.00	95.40	0	02:45	0.00		

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	91.41	76.96	191.27
Out-02	91.04	43.97	95.40
System	91.22	120.93	281.79

Link ID		Element	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
Ratio of Maximum	To Ti	tal Reported Type me Condition	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
MaxIllulli	11	me condicion	0ccu	rrence	Attained		Analysis	Capacity	/Design
Flow Surchar	ged						-		_
Damela min			days	hh:mm	m/sec		LPS	LPS	Flow
Depth min	utes								
 Link-01		CONDUIT	0	02:40	1.45	1.00	191.27	330.30	0.58
0.77	0	Calculated							
Link-02		CONDUIT	0	02:45	1.15	1.00	95.40	400.29	0.24
0.46	0	Calculated	0	00.00	0.00	1 00	0.00	006.00	0.00
Link-03 0.50	0	CHANNEL Calculated	0	00:00	0.00	1.00	0.00	926.98	0.00
Link-04	U	CHANNEL	0	00:00	0.00	1.00	0.00	243.76	0.00
0.00	0	Calculated	ŭ	00 00	0.00	1.00	0.00	213170	0.00
Link-05		CONDUIT	0	00:00	0.00	1.00	0.00	41.39	0.00
0.00	0	Calculated							
Link-06		CHANNEL	0	00:00	0.00	1.00	0.00	241.07	0.00

********* Highest Flow Instability Indexes All links are stable.

Analysis began on: Wed Nov 17 10:21:36 2021 Analysis ended on: Wed Nov 17 10:21:37 2021 Total elapsed time: 00:00:01

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-PRE-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ****** Number of rain gages 1 Number of subbasins 3 Number of nodes 9 Number of links 7 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min Rain Gage-01 25 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares _____

 Sub-01a
 5.53
 484.00

 Sub-01b
 1.24
 484.00

 Sub-02
 4.48
 484.00

 ******* Node Summary Element Invert Maximum Ponded External Type Elevation Elev. Area Inflow m m \mathfrak{m}^2 Node _____
 Jun-01
 JUNCTION
 198.90
 201.00
 0.00

 Jun-02
 JUNCTION
 199.84
 201.60
 0.00

 Jun-03
 JUNCTION
 201.60
 202.00
 0.00

 Jun-04
 JUNCTION
 202.46
 204.00
 0.00

 Jun-05
 JUNCTION
 202.48
 204.00
 0.00

 Jun-06
 JUNCTION
 202.72
 203.05
 0.00

Jun-07 Out-01 Out-02	JUNCTION OUTFALL OUTFALL		201.87 198.56 199.65	202.10 199.01 200.15	0.00 0.00 0.00		

Link ID	From Node	To Node		Element Type	Length m	૾	Manning's Roughness
Link-01 Link-02 Link-03 Link-04 Link-05 Link-06 Link-07	Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07	Out-01 Out-02 Jun-02 Jun-07 Jun-04 Jun-05 Jun-03		CONDUIT CONDUIT CHANNEL CHANNEL CONDUIT CHANNEL CHANNEL	19.0 12.7 10.0 63.2 8.2 21.0	1.7867 1.4961 17.6000 0.9328 0.2439	0.0150 0.0150 0.0320 0.0320 0.0320 0.0150 0.0320 0.0320

Cross Section							
Link	Shape	Depth/	,	Width	No. of	Cross	Full Flow
Design ID		Diameter	•		Barrels	Sectional	Hydraulic
Flow						Area	Radius
Capacity							Radius
LPS		n	1	m		m²	m
Link-01 330.30	CIRCULAR	0.45	5	0.45	1	0.16	0.11
Link-02 400.29	CIRCULAR	0.50)	0.50	1	0.20	0.12
Link-03	TRAPEZOIDAL	0.20)	1.95	1	0.27	0.13
926.98 Link-04	TRAPEZOIDAL	0.20)	2.90	1	0.34	0.12
243.76					1		
Link-05 41.39	CIRCULAR	0.30)	0.30	1	0.07	0.07
Link-06 241.07	TRAPEZOIDAL	0.20)	2.50	1	0.30	0.12
Link-07 201.32	TRAPEZOIDAL	0.10)	2.20	1	0.16	0.07
**************************************	ty Continuity	Volume	ı	Depth mm			
Total Precipi Surface Runof Continuity Er	tation f	0.758 0.638 -0.004	3	67.380 56.738			
************* Flow Routing *******	Continuity	Volume	ı	Volume Mliters			
External Infl External Outf Initial Store Final Stored Continuity Er	lowd Volume Volume	0.000 0.224 0.000 0.000 -0.000	<u> </u>	0.000 2.239 0.000 0.002			

Composite Curve Number Computations Report Subbasin Sub-01a Group Soil Area Area (ha) Soil/Surface Description CN 5.53 - 78.00 5.53 78.00 Composite Area & Weighted CN Subbasin Sub-01b Area Soil (ha) Group CN Soil/Surface Description - 73.40 73.40 1.24 Composite Area & Weighted CN 1.24 Subbasin Sub-02 ______ Soil Area Soil (ha) Group CN Soil/Surface Description ______ - 75.30 - 0.00 75.30 4.48 0.00 Composite Area & Weighted CN 4.48 Subbasin Runoff Summary Subbasin Total Total Peak Weighted Time of ID Precip Runoff Runoff Curve Concentration mm mm LPS Number days hh:mm:ss
 Sub-01a
 67.70
 22.78
 248.06
 78.000
 0 00:16:42

 Sub-01b
 67.70
 17.20
 43.33
 73.400
 0 00:08:05

 Sub-02
 67.70
 19.38
 151.50
 75.300
 0 00:21:20

| Node | Average | Maximum | Depth | Depth | Depth | Attained | Maximum | Maximum | Time of Max | Total | Total | Time | Time | Time | Attained | Maximum | Maximum | Maximum | Time of Max | Total | Total | Time |

Node ID	Element Type	Maximum Lateral	Peak Inflow		ime of	Maximum Flooding		f Peak
	1750	Inflow			rrence	Overflow		rrence
		LPS	LPS	days	hh:mm	LPS	days	hh:mm
Jun-01	JUNCTION	284.15	284.15	0	02:39	0.00		
Jun-02	JUNCTION	148.62	148.62	0	02:44	0.00		
				•				
Jun-03	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-04	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-05	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-06	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00		
Out-01	OUTFALL	0.00	284.10	0	02:39	0.00		
Out-02	OUTFALL	0.00	148.62	0	02:45	0.00		

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	92.86	109.17	284.10
Out-02	92.34	63.85	148.62
System	92.60	173.02	431.18

Link ID Ratio of		Element tal Reported	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
		Type	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
Maximum	Ti	me Condition	0ccu	rrence	Attained		Analysis	Capacity	/Design
Flow Surchar	ged						_		
			days	hh:mm	m/sec		LPS	LPS	Flow
Depth min	utes								
Link-01		CONDUIT	0	02:39	1.96	1.00	284.10	330.30	0.86
0.86	0	Calculated							
Link-02	_	CONDUIT	0	02:45	1.23	1.00	148.62	400.29	0.37
0.59	0	Calculated	•	00.00	0.00	1 00	0.00	006.00	0.00
Link-03	0	CHANNEL	0	00:00	0.00	1.00	0.00	926.98	0.00
0.50 Link-04	0	Calculated CHANNEL	0	00:00	0.00	1.00	0.00	243.76	0.00
0.00	0	Calculated	U	00.00	0.00	1.00	0.00	243.76	0.00
0.00 Link-05	U	CONDUIT	0	00:00	0.00	1.00	0.00	41.39	0.00
0.00	0	Calculated	U	00.00	0.00	1.00	0.00	41.39	0.00
Link-06	Ü	CHANNEL	0	00:00	0.00	1.00	0.00	241.07	0.00

25 Year Pre-Development Radbourne Severance

********* Highest Flow Instability Indexes

Link Link-02 (7) Link Link-01 (5)

Analysis began on: Wed Nov 17 10:23:31 2021 Analysis ended on: Wed Nov 17 10:23:33 2021 Total elapsed time: 00:00:02

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-PRE-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ****** Number of rain gages 1 Number of subbasins 3 Number of nodes 9 Number of links 7 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 100 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares _____

 Sub-01a
 5.53
 484.00

 Sub-01b
 1.24
 484.00

 Sub-02
 4.48
 484.00

 ******* Node Summary Element Invert Maximum Ponded External Type Elevation Elev. Area Inflow m m \mathfrak{m}^2 Node _____
 Jun-01
 JUNCTION
 198.90
 201.00
 0.00

 Jun-02
 JUNCTION
 199.84
 201.60
 0.00

 Jun-03
 JUNCTION
 201.60
 202.00
 0.00

 Jun-04
 JUNCTION
 202.46
 204.00
 0.00

 Jun-05
 JUNCTION
 202.48
 204.00
 0.00

 Jun-06
 JUNCTION
 202.72
 203.05
 0.00

Jun-07 Out-01 Out-02	JUNCTION OUTFALL OUTFALL		201.87 198.56 199.65	202.10 199.01 200.15	0.00 0.00 0.00		

Link ID	From Node	To Node		Element Type	Lengtl r	n *	Manning's Roughness
Link-01 Link-02 Link-03 Link-04 Link-05 Link-06 Link-07	Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07	Out-01 Out-02 Jun-02 Jun-07 Jun-04 Jun-05 Jun-03		CONDUIT CONDUIT CHANNEL CHANNEL CONDUIT CHANNEL CHANNEL	19.0 12.7 10.0 63.0 8.2 21.0	1.7867 1.4961 17.6000 0.9328 0.2439 1.1407	0.0150 0.0150 0.0320 0.0320 0.0320 0.0150 0.0320 0.0320
*******	*****						
Cross Section							
Link	Shape	Depth/		Width	No. of	Cross	Full Flow
Design ID		Diameter			Barrels	Sectional	Hydraulic
Flow						Area	Radius
Capacity							Raulus
LPS		m	I	m		m²	m
Link-01 330.30	CIRCULAR	0.45		0.45	1	0.16	0.11
Link-02	CIRCULAR	0.50		0.50	1	0.20	0.12
400.29 Link-03	TRAPEZOIDAL	0.20		1.95	1	0.27	0.13
926.98 Link-04	TRAPEZOIDAL	0.20		2.90	1	0.34	0.12
243.76 Link-05	CIRCULAR	0.30		0.30	1	0.07	0.07
41.39							
Link-06 241.07	TRAPEZOIDAL	0.20		2.50	1	0.30	0.12
Link-07 201.32	TRAPEZOIDAL	0.10		2.20	1	0.16	0.07
**************************************	ty Continuity	Volume hectare-m	L	Depth mm			
Total Precipi Surface Runof Continuity Er	f	0.926 0.952 -0.003		82.309 84.675			
*************** Flow Routing (Continuity	Volume hectare-m		Volume Mliters			
External Inflemental Store Final Stored Continuity Er	lowd Volume Volume	0.000 0.334 0.000 0.000 -0.000		0.000 3.344 0.000 0.003			

Subbasin Sub-01a							
Soil/Surface Des	cription				Area (ha)	Soil Group	CN
- Composite Area &	Weighted CN				5.53 5.53	-	78.00 78.00
Subbasin Sub-01b							
Soil/Surface Des	cription				Area (ha)	Soil Group	CN
- Composite Area &					1.24 1.24	-	73.40 73.40
Subbasin Sub-02							
Soil/Surface Des					Area (ha)	Soil Group	CN
	Weighted CN ****** Summary				4.48 0.00 4.48	-	75.30 0.00 75.30
Subbasin	Precip mm	Runoff mm	Runoff LPS	Weighted Curve Number	Cond days	centration hh:mm:ss	
Sub-01a Sub-01b Sub-02	82.70 82.70 82.70	33.38 26.44		78.000 73.400 75.300	0		

Node Depth Summary ********

Node	Average	Maximum	Maximum	Time	of Max	Total	Total	Retention
ID	Depth	Depth	HGL	Occi	ırrence	Flooded	Time	Time
	Attained	Attained	Attained			Volume	Flooded	
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.45	1.09	199.99	0	02:47	0	0	0:00:00
Jun-02	0.27	0.53	200.37	0	02:40	0	0	0:00:00
Jun-03	0.00	0.00	201.60	0	00:00	0	0	0:00:00
Jun-04	0.00	0.00	202.46	0	00:00	0	0	0:00:00
Jun-05	0.00	0.00	202.48	0	00:00	0	0	0:00:00
Jun-06	0.00	0.00	202.72	0	00:00	0	0	0:00:00
Jun-07	0.00	0.00	201.87	0	00:00	0	0	0:00:00
Out-01	0.21	0.45	199.01	0	02:27	0	0	0:00:00
Out-02	0.15	0.28	199.93	0	02:40	0	0	0:00:00

Node ID	Element Type	Maximum Lateral Inflow	Peak Inflow	Time of Peak Inflow Occurrence		Maximum Flooding Overflow	Time of Peak Flooding Occurrence	
		LPS	LPS				days hh:mm	
		пьр	LPS	uays	1111 • 111111	LPS	days IIII·	
Jun-01	JUNCTION	436.20	436.20	0	02:35	0.00		
Jun-02	JUNCTION	241.41	241.41	0	02:40	0.00		
Jun-03	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-04	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-05	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-06	JUNCTION	0.00	0.00	0	00:00	0.00		
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00		
Out-01	OUTFALL	0.00	436.30	0	02:35	0.00		
Out-02	OUTFALL	0.00	241.32	0	02:40	0.00		

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	94.00	164.01	436.30
Out-02	93.67	97.71	241.32
System	93.84	261.72	674.07

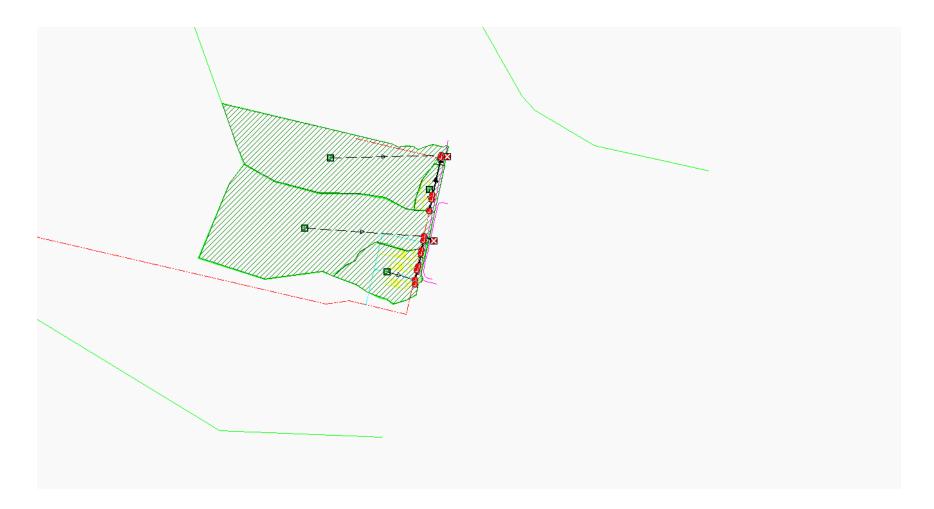
Link ID Ratio of		Element tal Reported	Ti	me of	Maximum	Length	Peak Flow	Design	Ratio of
Maximum		Type me Condition	Peak	Flow	Velocity	Factor	during	Flow	Maximum
	harged	me condition	Occur	rence	Attained		Analysis	Capacity	/Design
	minutes		days	hh:mm	m/sec		LPS	LPS	Flow
рерсп п	umuces								
Link-01		CONDUIT	0	02:35	2.74	1.00	436.30	330.30	1.32
1.00 Link-02	20	SURCHARGED CONDUIT	0	02:40	1.47	1.00	241.32	400.29	0.60
0.78	0	Calculated	ŭ	02 10		1.00	211.32	100.25	0.00
Link-03		CHANNEL	0	00:00	0.00	1.00	0.00	926.98	0.00
0.50	0	Calculated	0	00.00	0.00	1 00	0.00	0.42 56	0.00
Link-04 0.00	0	CHANNEL Calculated	0	00:00	0.00	1.00	0.00	243.76	0.00
Link-05	U	CONDUIT	0	00:00	0.00	1.00	0.00	41.39	0.00
0.00	0	Calculated							
Link-06		CHANNEL	0	00:00	0.00	1.00	0.00	241.07	0.00

********* Highest Flow Instability Indexes

Link Link-02 (22) Link Link-01 (5)

Analysis began on: Wed Nov 17 10:24:43 2021 Analysis ended on: Wed Nov 17 10:24:45 2021 Total elapsed time: 00:00:02

Post Development



CN Calculations

Post-Development

Area 1A:	CN	Area (m²)	CN*A
Impervious Areas (roadways, Buildings)	98	215.21	21090.58
Forest (Fair)	73	24752.47	1806930.31
Crop - Farm	82	30204.94	2476805.08
Grass	74	0.00	0.00
Gravel Driveway	89	120.26	10703.14
Total		55292.88	4315529.11
	CN		78.0

Area 1B:	CN	Area (m²)	CN*A
Impervious Areas (roadways, Buildings)	98	1672.87	163941.36
Forest (Fair)	73	8118.34	592638.75
Crop - Farm	82	0.00	0.00
Grass	74	2591.30	191756.20
Gravel Driveway	89	0.00	0.00
Total		12382.51	948336.31
	CN		76.6

Area 2:	CN	Area (m²)	CN*A
Impervious Areas (roadways)	98	132.04	12940.12
Forest (Woods)	73	22650.16	1653461.97
Cultivated	82	7402.56	607009.92
Grass	74	10825.54	801089.74
Gravel	89	290.59	25862.24
Total		41600.56	3100363.99
	CN		74.5

Area 2b:	CN	Area (m²)	CN*A
Impervious Areas (roadways)	98	814.50	79821.00
Forest (Woods)	73	2447.72	178683.56
Cultivated	82	0.00	0.00
Grass	74	0.00	0.00
Gravel	89	98.61	8776.29
Total		3205.80	267280.85
	CN		83.4

Soil Group C (Osprey Breypan)

Assume new buildings are 300m2

Driveways are 6m wide (asphalt)

Lot 1 driveway will be 43m long (33m in Dev env) and 2 driveway to be 32m long (25m in dev env)

Lot 3 driveway to be 18.2m long

Lot 4 driveway to consiste of 130m2 asphalt. The remainder to be gravel

		Imp Area in	
Lot No.	Dev Env	dev env	Grass
1	1744.0	498.00	1246.00
2	1335.5	450.00	885.5
3	827.0	367.20	459.80
	Total	1315.20	2591.30

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-post-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ******* Number of rain gages 1 Number of subbasins 4 Number of nodes 16 Number of links 14 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 2 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares -----5.53 484.00 1.24 484.00 4.16 484.00 0.32 484.00 Sub-01a Sub-01b Sub-02 Sub-04 ****** Node Summary ***** Node ID _____
 Jun-01
 JUNCTION
 198.90
 202.20
 0.00

 Jun-02
 JUNCTION
 201.48
 202.35
 0.00

 Jun-03
 JUNCTION
 201.55
 202.10
 0.00

 Jun-04
 JUNCTION
 201.75
 202.26
 0.00

 Jun-05
 JUNCTION
 202.33
 204.00
 0.00

Jun-06	JUNCTION	20	3.00	204.00	0.00		
Jun-07	JUNCTION		3.50		0.00		
Jun-08	JUNCTION			202.70	0.00		
Jun-09	JUNCTION	19	9.84	201.64	0.00		
Jun-10	JUNCTION	20	1.60	202.00	0.00		
Jun-11	JUNCTION			204.00			
Jun-12	JUNCTION			204.00			
Jun-13	JUNCTION			203.05			
Jun-14	JUNCTION						
Out-01	OUTFALL	19	8 56	202.10 199.01	0.00		
Out-02	OUTFALL	19	9 65	200.15	0.00		
040 02	OUTTIEL	10	J. 03	200.13	0.00		

Link Summary							

Link	From Node	To Node		Element	Length		
ID				Type		%	
CSP-1	Jun-07	Jun-06		CONDUIT	8.0	6.2657	0.0240
CSP-2	Jun-05	Jun-04		CONDUIT	8.0	7.2500	0.0240
CSP-3	Jun-05 Jun-03 Jun-01 Jun-08	Jun-02		CONDUIT	8.0	7.2500 0.8750 1.7867	0.0240
Link-01	Jun-01	Out-01 Jun-01		CONDUIT	19.0	1.7867	0.0150
Link-02	Jun-08			CHANNEL	7.9	32.6582	0.0300
Link-04	Jun-04	Jun-03		CHANNEL	20.0	1.0000 4.3620	0.0300
Link-06	Jun-06	Jun-05		CHANNEL	15.4	4.3620	0.0300
Link-08	Jun-02	Jun-08		CHANNEL	26.1	1.8391	0.0300
Link-09	Jun-09	Out-02		CONDUIT		1.6565	
Link-10	Jun-10	Jun-09		CHANNEL	10.0	17.6000	0.0320
Link-11	Jun-11	Jun-14		CHANNEL	63.2	0.9328	0.0320
Link-12	Jun-12	Jun-11		CONDUIT	8.7	0.2309	0.0240
Link-13	Jun-13	Jun-12		CHANNEL	21.0	1.1407	0.0320
Link-02 Link-04 Link-08 Link-09 Link-10 Link-11 Link-12 Link-13 Link-14	Jun-14	Jun-10		CHANNEL	5.5	0.9328 0.2309 1.1407 4.9091	0.0320
**************************************	Summary ******						
Link	Shape	Depth/		Width	No. of	Cross	Full Flow
Design							
ID		Diameter			Barrels	Sectional	Hydraulic
Flow						7	D = 44
Compaiter						Area	Radius
Capacity		m		m		m²	m
LPS		ш		ш		1111-	Ш
пго							
CSP-1	CIRCULAR	0.45		0.45	1	0.16	0.11
386.59							
CSP-2	CIRCULAR	0.45		0.45	1	0.16	0.11
415.85							
CSP-3	CIRCULAR	0.45		0.45	1	0.16	0.11
144.47							
Link-01	CIRCULAR	0.45		0.45	1	0.16	0.11
330.30							
Link-02	TRAPEZOIDAL	1.20		31.80	1	20.88	0.65
299865.48					_		
Link-04	TRAPEZOIDAL	0.45		3.00	1	0.84	0.27
1163.01				0 55			2 45
Link-06	TRAPEZOIDAL	1.00		2.75	1	1.75	0.49
7562.79				= 0.4	_		
Link-08	TRAPEZOIDAL	0.26		7.24	1	1.07	0.15
1351.97	a = p a = = = = =	2 = 2		0 50		2 22	2 4 5
Link-09	CIRCULAR	0.50		0.50	1	0.20	0.12
421.21							

Radbourne Severance 2 Year Post Devel						r Post Development	
Link-10	TRAPEZOIDAL	0.20	1.95		1	0.27	0.13
926.98 Link-11	TRAPEZOIDAL	0.20	3.40		1	0.44	0.13
337.34 Link-12	CIRCULAR	0.30	0.30		1	0.07	0.07
25.17 Link-13	TRAPEZOIDAL	0.20	3.00		1	0.40	0.13
345.42 Link-14 191.95	TRAPEZOIDAL	0.10	2.20		1	0.16	0.07
********	*****	Volume	Depth				
Runoff Quantity	y Continuity	hectare-m	mm 				
Total Precipita Surface Runoff Continuity Erro		0.416 0.143 -0.003	37.024 12.732				
**************************************	ontinuity	Volume hectare-m	Volume Mliters				
External Inflow External Outflow Initial Stored Final Stored Vo Continuity Erro	Volume	0.000 0.047 0.000 0.002 0.027	0.000 0.466 0.000 0.020				
Composite Curve		ations Report					
Subbasin Sub-01							
Soil/Surface De	escription			Area (ha)		Soil Group	CN
- Composite Area	& Weighted CN			5.53 5.53		-	78.00 78.00
Subbasin Sub-01	lb						
Soil/Surface De	_			Area (ha)		Soil Group	CN
- Composite Area	& Weighted CN			1.24 1.24		-	76.60 76.60
Subbasin Sub-02	2						
Soil/Surface De	-			,,		Soil Group	CN
- Composite Area	& Weighted CN			4.16 4.16		-	73.70 73.70
Subbasin Sub-04	1						
Soil/Surface De				Area (ha)		Soil Group	CN

- 0.32 - 83.40 Composite Area & Weighted CN 0.32 - 83.40

Subbasin ID	Total Precip mm	Total Runoff mm	Peak Runoff LPS	Weighted Curve Number	Time of Concentration days hh:mm:ss
Sub-01a	37.20	5.54	41.06	78.000	0 00:16:42
Sub-01b	37.20	4.72	8.50	76.600	0 00:08:05
Sub-02	37.20	3.33	12.18	73.700	0 00:21:20
Sub-04	37.20	9.45	7.08	83.400	0 00:01:38

Node ID	Average Depth Attained	Maximum Depth Attained	Maximum HGL Attained		of Max rrence	Total Flooded Volume	Total Time Flooded	Retention Time
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.10	0.16	199.06	0	02:45	0	0	0:00:00
Jun-02	0.01	0.02	201.50	0	02:42	0	0	0:00:00
Jun-03	0.06	0.09	201.64	0	02:41	0	0	0:00:00
Jun-04	0.02	0.03	201.78	0	02:40	0	0	0:00:00
Jun-05	0.05	0.09	202.42	0	02:40	0	0	0:00:00
Jun-06	0.01	0.02	203.02	0	02:40	0	0	0:00:00
Jun-07	0.00	0.00	203.50	0	00:00	0	0	0:00:00
Jun-08	0.31	0.48	201.48	0	04:07	0	0	0:00:00
Jun-09	0.09	0.12	199.96	0	04:00	0	0	0:00:00
Jun-10	0.01	0.01	201.61	0	02:37	0	0	0:00:00
Jun-11	0.02	0.03	202.49	0	02:33	0	0	0:00:00
Jun-12	0.06	0.10	202.58	0	02:35	0	0	0:00:00
Jun-13	0.01	0.02	202.74	0	02:30	0	0	0:00:00
Jun-14	0.01	0.02	201.89	0	02:37	0	0	0:00:00
Out-01	0.07	0.11	198.67	0	02:45	0	0	0:00:00
Out-02	0.05	0.06	199.71	0	04:00	0	0	0:00:00

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Peak	ime of Inflow rrence hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07 Jun-08 Jun-09	JUNCTION	40.64 0.00 0.00 0.00 0.00 7.98 0.00 0.00	40.64 7.80 7.90 7.91 7.97 7.98 0.00 7.77 14.21	0 0 0 0 0 0	02:45 02:41 02:40 02:40 02:40 02:39 00:00 02:42 04:00	0.00 0.00 0.00 0.00 0.00 0.00 0.00	

Jun-11	JUNCTION	0.00	7.31	0	02:32	0.00
Jun-12	JUNCTION	0.00	6.63	0	02:30	0.00
Jun-13	JUNCTION	6.67	6.67	0	02:30	0.00
Jun-14	JUNCTION	0.00	6.56	0	02:33	0.00
Out-01	OUTFALL	0.00	40.36	0	02:45	0.00
Out02	OUTFALL	0.00	14.20	0	04:00	0.00

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	85.45	23.87	40.36
Out-02	84.28	11.69	
System	84.86	35.57	52.89

Link ID Ratio of		Element Stal Reported	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
Maximum		Type .me Condition	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
		me Condition	0ccu	rrence	Attained		Analysis	Capacity	/Design
Flow Surcha	rged		days	hh:mm	m/sec		LPS	LPS	Flow
Depth mi	nutes.	:	_						
CSP-1		CONDUIT	0	00:00	0.00	1.00	0.00	386.59	0.00
0.02 CSP-2	0	Calculated CONDUIT	0	02:40	0.88	1.00	7.91	415.85	0.02
0.13	0	Calculated							
CSP-3 0.13	0	CONDUIT Calculated	0	02:41	0.66	1.00	7.80	144.47	0.05
0.13 Link-01	U	Calculated	0	02:45	1.03	1.00	40.36	330.30	0.12
0.29	0	Calculated							
Link-02		CHANNEL	0	04:07	0.02	1.00	4.65	299865.48	0.00
0.07	0	Calculated	0	00.40	0.21	1 00	7.00	1162 01	0.01
Link-04 0.14	0	CHANNEL Calculated	0	02:40	0.21	1.00	7.90	1163.01	0.01
Link-06	O	CHANNEL	0	02:40	0.22	1.00	7.97	7562.79	0.00
0.05	0	Calculated							
Link-08	_	CHANNEL	0	02:42	0.22	1.00	7.77	1351.97	0.01
0.53 Link-09	0	Calculated CONDUIT	0	04:00	0.57	1.00	14.20	421.21	0.03
0.19	0	Calculated	U	04.00	0.57	1.00	14.20	421.21	0.03
Link-10	Ü	CHANNEL	0	02:37	0.21	1.00	6.46	926.98	0.01
0.32	0	Calculated							
Link-11		CHANNEL	0	02:33	0.24	1.00	6.56	337.34	0.02
0.12	0	Calculated	0	02:32	0.67	1 00	U 21	05 17	0.29
Link-12 0.21	0	CONDUIT Calculated	0	02:32	0.67	1.00	7.31	25.17	0.29
Link-13	5	CHANNEL	0	02:30	0.11	1.00	6.63	345.42	0.02
0.30	0	Calculated							

Radbourne Severance 2 Year Post Development

Link-14 CHANNEL 0 02:37 0.43 1.00 6.46 191.95 0.03 0.14 0 Calculated

Highest Flow Instability Indexes All links are stable.

Analysis began on: Fri Nov 19 17:10:16 2021 Analysis ended on: Fri Nov 19 17:10:17 2021 Total elapsed time: 00:00:01

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-post-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ******* Number of rain gages 1 Number of subbasins 4 Number of nodes 16 Number of links 14 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 5 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares -----5.53 484.00 1.24 484.00 4.16 484.00 0.32 484.00 Sub-01a Sub-01b Sub-02 Sub-04 ****** Node Summary ***** Node ID _____
 Jun-01
 JUNCTION
 198.90
 202.20
 0.00

 Jun-02
 JUNCTION
 201.48
 202.35
 0.00

 Jun-03
 JUNCTION
 201.55
 202.10
 0.00

 Jun-04
 JUNCTION
 201.75
 202.26
 0.00

 Jun-05
 JUNCTION
 202.33
 204.00
 0.00

Jun-06 Jun-07 Jun-08 Jun-09 Jun-10 Jun-11 Jun-12 Jun-13 Jun-14 Out-01 Out-02	JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL OUTFALL	203.00 203.50 201.00 199.84 201.60 202.46 202.48 202.72 201.87 198.56	204.00 202.70 201.64 202.00 204.00 204.00 203.05 202.10	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0		
Link Summary *********** Link ID	From Node	To Node	Element Type	Length m	ે	Roughness
CSP-1 CSP-2 CSP-3 Link-01 Link-02 Link-04 Link-06 Link-08 Link-09 Link-10 Link-11 Link-11 Link-12 Link-13 Link-14 ***********************************	Summary	Jun-06 Jun-04 Jun-02 Out-01 Jun-03 Jun-05 Jun-08 Out-02 Jun-09 Jun-14 Jun-11 Jun-12 Jun-10	CONDUIT CONDUIT CONDUIT CONDUIT CHANNEL	8.0 8.0 8.0 19.0 7.9 20.0 15.4 26.1 11.5 10.0 63.2 8.7 21.0 5.5	7.2500 0.8750 1.7867 32.6582 1.0000 4.3620 1.8391 1.6565 17.6000 0.9328 0.2309 1.1407 4.9091	0.0240 0.0240 0.0240 0.0150 0.0300 0.0300 0.0300 0.0320 0.0320 0.0320 0.0320 0.0320 0.0320
Flow Capacity					Area	Radius
LPS		m	m		m ²	m
CSP-1	CIRCULAR	0.45	0.45	1	0.16	0.11
386.59 CSP-2 415.85	CIRCULAR	0.45	0.45	1	0.16	0.11
CSP-3	CIRCULAR	0.45	0.45	1	0.16	0.11
144.47 Link-01	CIRCULAR	0.45	0.45	1	0.16	0.11
330.30 Link-02	TRAPEZOIDAL	1.20	31.80	1	20.88	0.65
299865.48 Link-04	TRAPEZOIDAL	0.45	3.00	1	0.84	0.27
1163.01 Link-06	TRAPEZOIDAL	1.00	2.75	1	1.75	0.49
7562.79 Link-08	TRAPEZOIDAL	0.26	7.24	1	1.07	0.15
1351.97 Link-09 421.21	CIRCULAR	0.50	0.50	1	0.20	0.12

Radbourne Severance						5 Year	Post Deve	evelopment	
Link-10 TF 926.98	RAPEZOIDAL	0.20	1.95		1	0.27		0.13	
Link-11 TF 337.34	RAPEZOIDAL	0.20	3.40		1	0.44		0.13	
	IRCULAR	0.30	0.30		1	0.07		0.07	
	RAPEZOIDAL	0.20	3.00		1	0.40		0.13	
	RAPEZOIDAL	0.10	2.20		1	0.16		0.07	
**************************************		Volume hectare-m	Depth mm						
Runoff Quantity Cor	*****								
Total Precipitation Surface Runoff Continuity Error (%		0.553 0.313 -0.004	49.167 27.826						
**************************************	nuity	Volume hectare-m	Volume Mliters						
External Inflow External Outflow Initial Stored Volume Final Stored Volume Continuity Error (%	ime	0.000 0.106 0.000 0.002	0.000 1.059 0.000 0.023						
Subbasin Sub-01a									
Soil/Surface Descri	iption			Area (ha)		Soil Group	CN		
- Composite Area & We	eighted CN			5.53 5.53		-	78.00 78.00		
Subbasin Sub-01b									
Soil/Surface Descri	lption					Soil Group	CN		
- Composite Area & We	eighted CN			1.24 1.24		-	76.60 76.60		
Subbasin Sub-02									
Soil/Surface Descri				Area (ha)		Soil Group	CN		
- Composite Area & We	eighted CN			4.16 4.16		-	73.70 73.70		
Subbasin Sub-04									
Soil/Surface Descri				Area (ha)		Soil Group	CN		

- 0.32 - 83.40 Composite Area & Weighted CN 0.32 - 83.40

Subbasin	Total Precip mm	Total Runoff mm	Peak Runoff LPS	Weighted Curve Number	Conc days	Time of entration hh:mm:ss
Sub-01a	49.40	11.53	111.00	78.000	0	00:16:42
Sub-01b	49.40	10.29	24.35	76.600	0	00:08:05
Sub-02	49.40	8.03	43.89	73.700	0	00:21:20
Sub-04	49.40	17.17	13.59	83.400	0	00:01:38

Node	Average	Maximum	Maximum	Time	of Max	Total	Total	Retention
ID	Depth	Depth	HGL	Occi	ırrence	Flooded	Time	Time
	Attained	Attained	Attained			Volume	Flooded	
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.20	0.36	199.26	0	02:41	0	0	0:00:00
Jun-02	0.02	0.04	201.52	0	02:37	0	0	0:00:00
Jun-03	0.09	0.13	201.68	0	02:36	0	0	0:00:00
Jun-04	0.03	0.06	201.81	0	02:35	0	0	0:00:00
Jun-05	0.08	0.11	202.44	0	02:40	0	0	0:00:00
Jun-06	0.02	0.04	203.04	0	02:35	0	0	0:00:00
Jun-07	0.00	0.00	203.50	0	00:00	0	0	0:00:00
Jun-08	0.39	0.49	201.49	0	02:53	0	0	0:00:00
Jun-09	0.12	0.17	200.01	0	02:45	0	0	0:00:00
Jun-10	0.01	0.02	201.62	0	02:34	0	0	0:00:00
Jun-11	0.02	0.05	202.51	0	02:33	0	0	0:00:00
Jun-12	0.07	0.12	202.60	0	02:31	0	0	0:00:00
Jun-13	0.02	0.03	202.75	0	02:30	0	0	0:00:00
Jun-14	0.01	0.02	201.89	0	02:34	0	0	0:00:00
Out-01	0.11	0.18	198.74	0	02:41	0	0	0:00:00
Out-02	0.08	0.12	199.77	0	02:45	0	0	0:00:00

Node	Element	Maximum	Peak		ime of	Maximum			
ID	Type	Lateral	Inflow		Inflow	Flooding		oding	
		Inflow		Occurrence		Overflow	Occur:	Occurrence	
		LPS	LPS	days	hh:mm	LPS	days :	hh:mm	
Jun-01	JUNCTION	110.99	110.99	0	02:40	0.00			
Jun-02	JUNCTION	0.00	23.88	0	02:36	0.00			
Jun-03	JUNCTION	0.00	23.99	0	02:35	0.00			
Jun-04	JUNCTION	0.00	24.08	0	02:40	0.00			
Jun-05	JUNCTION	0.00	24.17	0	02:35	0.00			
Jun-06	JUNCTION	24.22	24.22	0	02:35	0.00			
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00			
Jun-08	JUNCTION	0.00	23.78	0	02:37	0.00			
Jun-09	JUNCTION	43.60	51.04	0	02:45	0.00			
Jun-10	JUNCTION	0.00	12.48	0	02:34	0.00			

Jun-11	JUNCTION	0.00	12.56	0	02:31	0.00
Jun-12	JUNCTION	0.00	12.75	0	02:30	0.00
Jun-13	JUNCTION	12.80	12.80	0	02:30	0.00
Jun-14	JUNCTION	0.00	12.50	0	02:33	0.00
Out-01	OUTFALL	0.00	108.44	0	02:41	0.00
Out-02	OUTFALL	0.00	50.90	0	02:45	0.00

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	91.50	53.35	108.44 50.90
Out-02	90.77	27.88	
System	91.13	81.22	157.61

Link ID Ratio of		Element Stal Reported	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
		Type	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
	Maximum Time Condition		Occu	rrence	Attained		Analysis	Capacity	/Design
Flow Surcharged		days	hh:mm	m/sec		LPS	LPS	Flow	
Depth minutes		-							
CSP-1 0.04	0	CONDUIT Calculated	0	00:00	0.00	1.00	0.00	386.59	0.00
CSP-2	0	CONDUIT	0	02:40	1.19	1.00	24.08	415.85	0.06
0.19 CSP-3	0	Calculated CONDUIT	0	02:36	1.13	1.00	23.88	144.47	0.17
0.19	0	Calculated	U	02.30	1.13	1.00	23.00	144.47	0.17
Link-01 0.59	0	CONDUIT Calculated	0	02:41	1.16	1.00	108.44	330.30	0.33
Link-02	U	CHANNEL	0	02:53	0.02	1.00	14.69	299865.48	0.00
0.15 Link-04	0	Calculated CHANNEL	0	02:35	0.25	1.00	23.99	1163.01	0.02
0.21	0	Calculated	U				23.99	1103.01	
Link-06 0.08	0	CHANNEL Calculated	0	02:35	0.38	1.00	24.17	7562.79	0.00
Link-08	U	CHANNEL	0	02:37	0.23	1.00	23.78	1351.97	0.02
0.57 Link-09	0	Calculated CONDUIT	0	02:45	1.08	1.00	50.90	421.21	0.12
0.29	0	Calculated	0						
Link-10 0.46	0	CHANNEL Calculated	0	02:34	0.21	1.00	12.48	926.98	0.01
Link-11	U	CHANNEL	0	02:33	0.30	1.00	12.50	337.34	0.04
0.17 Link-12	0	Calculated CONDUIT	0	02:31	0.78	1.00	12.56	25.17	0.50
0.28	0	Conduit	U	UZ•31	0.78	1.00	12.50	25.17	0.50
Link-13 0.39	0	CHANNEL Calculated	0	02:30	0.12	1.00	12.75	345.42	0.04
0.39	U	Calculated							

Radbourne Severance 5 Year Post Development

Link-14 CHANNEL 0 02:34 0.54 1.00 12.48 191.95 0.06 0.20 0 Calculated

Link Link-09 (66) Link Link-01 (15) Link CSP-2 (11) Link Link-04 (10) Link CSP-3 (7)

Analysis began on: Fri Nov 19 17:11:06 2021
Analysis ended on: Fri Nov 19 17:11:07 2021

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-post-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ******* Number of rain gages 1 Number of subbasins 4 Number of nodes 16 Number of links 14 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 10 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares -----5.53 484.00 1.24 484.00 4.16 484.00 0.32 484.00 Sub-01a Sub-01b Sub-02 Sub-04 ****** Node Summary ***** Node ID _____
 Jun-01
 JUNCTION
 198.90
 202.20
 0.00

 Jun-02
 JUNCTION
 201.48
 202.35
 0.00

 Jun-03
 JUNCTION
 201.55
 202.10
 0.00

 Jun-04
 JUNCTION
 201.75
 202.26
 0.00

 Jun-05
 JUNCTION
 202.33
 204.00
 0.00

Jun-06 Jun-07 Jun-08 Jun-09 Jun-10 Jun-11 Jun-12 Jun-13 Jun-14 Out-01 Out-02	JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION JUNCTION OUTFALL OUTFALL	203.00 203.50 201.00 199.84 201.60 202.46 202.48 202.72 201.87 198.56 199.65	204.00 204.00 202.70 201.64 202.00 204.00 203.05 202.10 199.01 200.15	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0		
Link Summary *******						
Link ID	From Node	To Node	Element Type	Length m	- %	_
CSP-1	Jun-07	Jun-06	CONDUIT	8.0	6.2657	0.0240
CSP-2	Jun-05	Jun-04	CONDUIT	8.0		0.0240
CSP-3	Jun-03	Jun-02	CONDUIT	8.0		0.0210
Link-01	Jun-01	Out-01	CONDUIT	19.0		0.0150
Link-02	Jun-08	Jun-01	CHANNEL	7.9		0.0300
Link-04	Jun-04	Jun-03	CHANNEL	20.0		0.0300
Link-06	Jun-06	Jun-05	CHANNEL	15.4		0.0300
Link-08	Jun-02	Jun-08	CHANNEL	26.1		0.0300
Link-09	Jun-09	Out-02	CONDUIT	11.5		0.0150
Link-10	Jun-10	Jun-09	CHANNEL	10.0		0.0320
Link-11	Jun-11	Jun-14	CHANNEL	63.2		0.0320
Link-12	Jun-12	Jun-11	CONDUIT	8.7		0.0240
Link-13	Jun-13	Jun-12	CHANNEL	21.0		0.0320
Link-14	Jun-14	Jun-10	CHANNEL	5.5	4.9091	0.0320
********** Cross Section ******* Link Design ID Flow Capacity	Summary	Depth/ Diameter	Width	No. of Barrels	Cross Sectional Area	Full Flow Hydraulic Radius
		m	m		m²	m
LPS						
CSP-1	CIRCULAR	0.45	0.45	1	0.16	0.11
386.59						
CSP-2	CIRCULAR	0.45	0.45	1	0.16	0.11
415.85						
CSP-3	CIRCULAR	0.45	0.45	1	0.16	0.11
144.47						
Link-01 330.30	CIRCULAR	0.45	0.45	1	0.16	0.11
Link-02 299865.48	TRAPEZOIDAL	1.20	31.80	1	20.88	0.65
Link-04 1163.01	TRAPEZOIDAL	0.45	3.00	1	0.84	0.27
Link-06 7562.79	TRAPEZOIDAL	1.00	2.75	1	1.75	0.49
Link-08 1351.97	MD A DE COTDAT	0.26	7.24	1	1.07	0.15
	TRAPEZOIDAL	0.20	7.21	_		0.13
Link-09	CIRCULAR	0.50	0.50	1	0.20	0.13

Radbourne Severanc	е					10 Yea	r Post Deve	lopment
Link-10 926.98	TRAPEZOIDAL	0.20	1.95		1	0.27	С	1.13
Link-11 337.34	TRAPEZOIDAL	0.20	3.40		1	0.44	С	1.13
Link-12 25.17	CIRCULAR	0.30	0.30		1	0.07	C	0.07
Link-13 345.42	TRAPEZOIDAL	0.20	3.00		1	0.40	C	.13
Link-14 191.95	TRAPEZOIDAL	0.10	2.20		1	0.16	C	0.07
**************************************	y Continuity	Volume hectare-m	Depth mm					
Total Precipit Surface Runoff Continuity Err		0.643 0.447 -0.004	57.129 39.775					
**************************************	ontinuity	Volume hectare-m	Volume Mliters					
External Inflo External Outfl Initial Stored Final Stored V Continuity Err	ow	0.000 0.153 0.000 0.002 0.010	0.000 1.529 0.000 0.025					
Composite Curv		ations Report						
				Area	ç	Soil		
Soil/Surface D	escription			(ha)		roup	CN	
=	& Weighted CN			5.53 5.53		-	78.00 78.00	
Subbasin Sub-0								
Soil/Surface D					Gı	_	CN	
- Composite Area	& Weighted CN			1.24 1.24		-	76.60 76.60	
Subbasin Sub-0	2							
Soil/Surface D	escription			Area (ha)	Gı	Soil coup 	CN	
- Composite Area	& Weighted CN			4.16 4.16		-	73.70 73.70	
Subbasin Sub-0	4							
Soil/Surface D	escription			Area (ha)		Soil coup	CN	

- 0.32 - 83.40 Composite Area & Weighted CN 0.32 - 83.40

Subbasin ID	Total Precip mm	Total Runoff mm	Peak Runoff LPS	Weighted Curve Number	Time of Concentration days hh:mm:ss
Sub-01a	57.40	16.18	167.35	78.000	0 00:16:42
Sub-01b	57.40	14.68	37.10	76.600	0 00:08:05
Sub-02	57.40	11.86	74.76	73.700	0 00:21:20
Sub-04	57.40	22.86	18.41	83.400	0 00:01:38

Node ID	Average Depth Attained	Maximum Depth Attained	Maximum HGL Attained	Time of Max Occurrence		Total Flooded Volume	Total Time Flooded	Retention Time
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.25	0.54	199.44	0	02:42	0	0	0:00:00
Jun-02	0.02	0.05	201.53	0	02:36	0	0	0:00:00
Jun-03	0.10	0.16	201.71	0	02:36	0	0	0:00:00
Jun-04	0.04	0.08	201.83	0	02:35	0	0	0:00:00
Jun-05	0.09	0.14	202.47	0	02:35	0	0	0:00:00
Jun-06	0.03	0.05	203.05	0	02:35	0	0	0:00:00
Jun-07	0.00	0.00	203.50	0	00:00	0	0	0:00:00
Jun-08	0.41	0.49	201.49	0	02:40	0	0	0:00:00
Jun-09	0.15	0.25	200.09	0	02:46	0	0	0:00:00
Jun-10	0.01	0.02	201.62	0	02:33	0	0	0:00:00
Jun-11	0.03	0.05	202.51	0	02:32	0	0	0:00:00
Jun-12	0.08	0.14	202.62	0	02:31	0	0	0:00:00
Jun-13	0.02	0.04	202.76	0	02:30	0	0	0:00:00
Jun-14	0.01	0.03	201.90	0	02:33	0	0	0:00:00
Out-01	0.13	0.25	198.81	0	02:42	0	0	0:00:00
Out-02	0.09	0.15	199.80	0	02:46	0	0	0:00:00

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Peak Occu	ime of Inflow rrence hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
Jun-01 Jun-02 Jun-03 Jun-04 Jun-05 Jun-06 Jun-07 Jun-08 Jun-09 Jun-10	JUNCTION	167.34 0.00 0.00 0.00 0.00 37.11 0.00 0.00 74.63	201.07 36.70 36.87 36.93 37.06 37.11 0.00 36.60 84.39	0 0 0 0 0 0 0	02:40 02:36 02:35 02:35 02:35 02:35 00:00 02:36 02:40	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0	

Jun-11	JUNCTION	0.00	17.00	0	02:31	0.00
Jun-12	JUNCTION	0.00	17.19	0	02:30	0.00
Jun-13	JUNCTION	17.23	17.23	0	02:29	0.00
Jun-14	JUNCTION	0.00	16.93	0	02:32	0.00
Out-01	OUTFALL	0.00	190.23	0	02:42	0.00
Out-02	OUTFALL	0.00	85.39	0	02:46	0.00

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	92.71	76.13	190.23
Out-02	92.14	40.54	85.39
System	92.43	 116.67	275.17

Link ID		Element	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
Ratio of	To	tal Reported	Daa	l- 171	Velocity	T	during	Flow	Maximum
Maximum	Тi	Type .me Condition	Реа	K FIOW	velocity	Factor	during	FIOW	Maximum
Hazirmanı	Time Condition		0ccu	rrence	Attained		Analysis	Capacity	/Design
Flow Surch	arged								
			days	hh:mm	m/sec		LPS	LPS	Flow
Depth m	Depth minutes								
CSP-1		CONDUIT	0	00:00	0.00	1.00	0.00	386.59	0.00
0.06	0		0	00.25	1 00	1 00	26.02	415 05	0.00
CSP-2 0.24	0	CONDUIT Calculated	0	02:35	1.23	1.00	36.93	415.85	0.09
CSP-3	U	CONDUIT	0	02:36	1.28	1.00	36.70	144.47	0.25
0.24	0	Calculated	· ·	02.50	1.20	1.00	30.70	111.17	0.23
Link-01		CONDUIT	0	02:42	1.44	1.00	190.23	330.30	0.58
0.77	0	Calculated							
Link-02		CHANNEL	0	02:40	0.02	1.00	33.75	299865.48	0.00
0.23	0	Calculated							
Link-04	_	CHANNEL	0	02:35	0.29	1.00	36.87	1163.01	0.03
0.27	0	Calculated	0	00.05	0.45	1 00	25.06	5560 50	0.00
Link-06 0.10	0	CHANNEL Calculated	0	02:35	0.45	1.00	37.06	7562.79	0.00
Link-08	U	CAICUIACEG	0	02:36	0.23	1.00	36.60	1351.97	0.03
0.59	0	Calculated	U	02.30	0.23	1.00	30.00	1331.97	0.03
Link-09	Ü	CONDUIT	0	02:46	1.17	1.00	85.39	421.21	0.20
0.40	0	Calculated							
Link-10		CHANNEL	0	02:33	0.20	1.00	16.90	926.98	0.02
0.55	0	Calculated							
Link-11		CHANNEL	0	02:32	0.34	1.00	16.93	337.34	0.05
0.20	0	Calculated							
Link-12		CONDUIT	0	02:31	0.86	1.00	17.00	25.17	0.68
0.32 Link-13	0	Calculated CHANNEL	0	02:30	0.13	1.00	17 10	245 40	0.05
0.45	0	CHANNEL	0	02.30	0.13	1.00	17.19	345.42	0.05
0.40	U	carcurated							

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Link-14 CHANNEL 0 02:33 0.60 1.00 16.90 191.95 0.09 0.25 0 Calculated

Link Link-09 (34) Link CSP-3 (20) Link CSP-2 (18) Link Link-08 (17) Link Link-04 (17)

Analysis began on: Sat Nov 20 15:06:47 2021 Analysis ended on: Sat Nov 20 15:06:48 2021

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-post-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ******* Number of rain gages 1 Number of subbasins 4 Number of nodes 16 Number of links 14 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 25 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares -----5.53 484.00 1.24 484.00 4.16 484.00 0.32 484.00 Sub-01a Sub-01b Sub-02 Sub-04 ****** Node Summary ***** Node ID _____
 Jun-01
 JUNCTION
 198.90
 202.20
 0.00

 Jun-02
 JUNCTION
 201.48
 202.35
 0.00

 Jun-03
 JUNCTION
 201.55
 202.10
 0.00

 Jun-04
 JUNCTION
 201.75
 202.26
 0.00

 Jun-05
 JUNCTION
 202.33
 204.00
 0.00

Jun-06	JUNCTION	20	3.00	204.00	0.00		
Jun-07	JUNCTION		3.50		0.00		
Jun-08	JUNCTION			202.70	0.00		
Jun-09	JUNCTION	19	9.84	201.64	0.00		
Jun-10	JUNCTION	20	1.60	202.00	0.00		
Jun-11	JUNCTION			204.00			
Jun-12	JUNCTION			204.00			
Jun-13	JUNCTION			203.05			
Jun-14	JUNCTION						
Out-01	OUTFALL	19	8 56	202.10 199.01	0.00		
Out-02	OUTFALL	19	9 65	200.15	0.00		
040 02	OUTTIEL	10	J. 03	200.13	0.00		

Link Summary							

Link	From Node	To Node		Element	Length		
ID				Type		%	
CSP-1	Jun-07	Jun-06		CONDUIT	8.0	6.2657	0.0240
CSP-2	Jun-05	Jun-04		CONDUIT	8.0	7.2500	0.0240
CSP-3	Jun-05 Jun-03 Jun-01 Jun-08	Jun-02		CONDUIT	8.0	7.2500 0.8750 1.7867	0.0240
Link-01	Jun-01	Out-01 Jun-01		CONDUIT	19.0	1.7867	0.0150
Link-02	Jun-08			CHANNEL	7.9	32.6582	0.0300
Link-04	Jun-04	Jun-03		CHANNEL	20.0	1.0000 4.3620	0.0300
Link-06	Jun-06	Jun-05		CHANNEL	15.4	4.3620	0.0300
Link-08	Jun-02	Jun-08		CHANNEL	26.1	1.8391	0.0300
Link-09	Jun-09	Out-02		CONDUIT		1.6565	
Link-10	Jun-10	Jun-09		CHANNEL	10.0	17.6000	0.0320
Link-11	Jun-11	Jun-14		CHANNEL	63.2	0.9328	0.0320
Link-12	Jun-12	Jun-11		CONDUIT	8.7	0.2309	0.0240
Link-13	Jun-13	Jun-12		CHANNEL	21.0	1.1407	0.0320
Link-02 Link-04 Link-08 Link-09 Link-10 Link-11 Link-12 Link-13 Link-14	Jun-14	Jun-10		CHANNEL	5.5	0.9328 0.2309 1.1407 4.9091	0.0320
**************************************	Summary ******						
Link	Shape	Depth/		Width	No. of	Cross	Full Flow
Design							
ID		Diameter			Barrels	Sectional	Hydraulic
Flow						7	D = 44
Compaiter						Area	Radius
Capacity		m		m		m²	m
LPS		ш		ш		1111-	Ш
пго							
CSP-1	CIRCULAR	0.45		0.45	1	0.16	0.11
386.59							
CSP-2	CIRCULAR	0.45		0.45	1	0.16	0.11
415.85							
CSP-3	CIRCULAR	0.45		0.45	1	0.16	0.11
144.47							
Link-01	CIRCULAR	0.45		0.45	1	0.16	0.11
330.30							
Link-02	TRAPEZOIDAL	1.20		31.80	1	20.88	0.65
299865.48					_		
Link-04	TRAPEZOIDAL	0.45		3.00	1	0.84	0.27
1163.01				0 55			2 45
Link-06	TRAPEZOIDAL	1.00		2.75	1	1.75	0.49
7562.79				= 0.4	_		
Link-08	TRAPEZOIDAL	0.26		7.24	1	1.07	0.15
1351.97	a = p a = = = = =	2 = 2		0 50		2 22	2 4 5
Link-09	CIRCULAR	0.50		0.50	1	0.20	0.12
421.21							

Radbourne Severance					25 Y	ear Post Development
Link-10	TRAPEZOIDAL	0.20	1.95		1 0.2	7 0.13
926.98 Link-11	TRAPEZOIDAL	0.20	3.40		1 0.4	4 0.13
337.34 Link-12	CIRCULAR	0.30	0.30		1 0.0	7 0.07
25.17 Link-13	TRAPEZOIDAL	0.20	3.00		1 0.4	0 0.13
345.42 Link-14 191.95	TRAPEZOIDAL	0.10	2.20		1 0.1	6 0.07
**************************************	Continuity	Volume hectare-m	Depth mm			
Total Precipitat Surface Runoff . Continuity Error	ion	0.758 0.641 -0.004	67.380 57.000			
**************************************	ntinuity	Volume hectare-m	Volume Mliters			
External Inflow External Outflow Initial Stored V Final Stored Vol Continuity Error	v Volume Lume	0.000 0.221 0.000 0.003 0.006	0.000 2.209 0.000 0.027			
**************************************	Number Computa	ations Report				
				Area	Soil	
Soil/Surface Des	scription 			(ha) 	Group 	CN
Composite Area &				5.53 5.53	-	78.00 78.00
Subbasin Sub-01k						
Soil/Surface Des	scription			Area (ha)	Soil Group	CN
- Composite Area &				1.24 1.24	-	76.60 76.60
Subbasin Sub-02						
				Area	Soil	
Soil/Surface Des	scription			(ha) 	Group	CN
- Composite Area &	Weighted CN			4.16 4.16	-	73.70 73.70
Subbasin Sub-04						
				Area	Soil	

- 0.32 - 83.40 Composite Area & Weighted CN 0.32 - 83.40

Subbasin ID	Total Precip mm	Total Runoff mm	Peak Runoff LPS	Weighted Curve Number	Time of Concentration days hh:mm:ss
Sub-01a	67.70	22.78	248.06	78.000	0 00:16:42
Sub-01b	67.70	20.98	56.07	76.600	0 00:08:05
Sub-02	67.70	17.53	122.33	73.700	0 00:21:20
Sub-04	67.70	30.66	24.92	83.400	0 00:01:38

Node ID	Average Depth Attained	Maximum Depth Attained	Maximum HGL Attained	Time of Max Occurrence		Total Flooded Volume	Total Time Flooded	Retention Time
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.33	0.81	199.71	0	02:42	0	0	0:00:00
Jun-02	0.03	0.06	201.54	0	02:35	0	0	0:00:00
Jun-03	0.12	0.21	201.76	0	02:32	0	0	0:00:00
Jun-04	0.05	0.10	201.85	0	02:35	0	0	0:00:00
Jun-05	0.11	0.19	202.52	0	02:32	0	0	0:00:00
Jun-06	0.03	0.07	203.07	0	02:35	0	0	0:00:00
Jun-07	0.00	0.00	203.50	0	00:00	0	0	0:00:00
Jun-08	0.42	0.50	201.50	0	02:35	0	0	0:00:00
Jun-09	0.19	0.36	200.20	0	02:40	0	0	0:00:00
Jun-10	0.01	0.03	201.63	0	02:32	0	0	0:00:00
Jun-11	0.03	0.06	202.52	0	02:31	0	0	0:00:00
Jun-12	0.09	0.16	202.64	0	02:30	0	0	0:00:00
Jun-13	0.02	0.05	202.77	0	02:30	0	0	0:00:00
Jun-14	0.02	0.03	201.90	0	02:32	0	0	0:00:00
Out-01	0.16	0.32	198.88	0	02:42	0	0	0:00:00
Out-02	0.11	0.20	199.85	0	02:40	0	0	0:00:00

Node ID	Element Type	Maximum Lateral Inflow	Peak Inflow	Peak	ime of Inflow	Maximum Flooding Overflow	Time of Peak Flooding Occurrence
		LPS	LPS	days	hh:mm	LPS	days hh:mm
Jun-01	JUNCTION	247.16	296.03	0	02:39	0.00	
Jun-02	JUNCTION	0.00	57.15	0	02:35	0.00	
Jun-03	JUNCTION	0.00	55.31	0	02:35	0.00	
Jun-04	JUNCTION	0.00	55.59	0	02:32	0.00	
Jun-05	JUNCTION	0.00	55.42	0	02:35	0.00	
Jun-06	JUNCTION	55.45	55.45	0	02:34	0.00	
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-08	JUNCTION	0.00	55.45	0	02:35	0.00	
Jun-09	JUNCTION	121.03	137.44	0	02:40	0.00	
Jun-10	JUNCTION	0.00	22.95	0	02:32	0.00	

Jun-11	JUNCTION	0.00	23.05	0	02:30	0.00
Jun-12	JUNCTION	0.00	23.20	0	02:30	0.00
Jun-13	JUNCTION	23.23	23.23	0	02:29	0.00
Jun-14	JUNCTION	0.00	22.98	0	02:31	0.00
Out-01	OUTFALL	0.00	282.86	0	02:42	0.00
Out-02	OUTFALL	0.00	137.05	0	02:40	0.00

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	93.75	108.49	282.86
Out-02	93.21	59.30	137.05
System	93 48	167 79	419 04

Link ID Ratio of		Element Stal Reported	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
		Type	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
Maximum	mum Time Condition		0ccu	rrence	Attained		Analysis	Capacity	/Design
Flow Surch	arged		days	hh:mm	m/sec		LPS	LPS	Flow
Depth m	inutes	3	-						
CSP-1 0.07	0	CONDUIT Calculated	0	00:00	0.00	1.00	0.00	386.59	0.00
CSP-2	U	CONDUIT	0	02:32	1.36	1.00	55.59	415.85	0.13
0.32 CSP-3	0	Calculated CONDUIT	0	02:35	1.45	1.00	57.15	144.47	0.40
0.30	0	Calculated	U	02.35	1.45	1.00	57.15	144.4/	0.40
Link-01 0.86	0	CONDUIT Calculated	0	02:42	1.95	1.00	282.86	330.30	0.86
Link-02	U	CHANNEL	0	02:35	0.03	1.00	55.17	299865.48	0.00
0.34 Link-04	0	Calculated CHANNEL	0	02:35	0.33	1.00	55.31	1163.01	0.05
0.34	0	Calculated	U	02.33	0.33	1.00	55.31	1103.01	0.05
Link-06 0.13	0	CHANNEL Calculated	0	02:35	0.51	1.00	55.42	7562.79	0.01
Link-08	U	CHANNEL	0	02:35	0.22	1.00	55.45	1351.97	0.04
0.61 Link-09	0	Calculated CONDUIT	0	02:40	1.22	1.00	137.05	421.21	0.33
0.56	0	Calculated	U	02.40	1.22	1.00	137.03	421.21	0.33
Link-10 0.56	0	CHANNEL Calculated	0	02:32	0.21	1.00	22.95	926.98	0.02
Link-11	U	CHANNEL	0	02:31	0.38	1.00	22.98	337.34	0.07
0.24 Link-12	0	Calculated	٥	02:30	0.95	1.00	22.05	25 17	0.92
0.38	0	CONDUIT Calculated	0	02.30	0.95	1.00	23.05	25.17	0.92
Link-13	0	CHANNEL	0	02:30	0.15	1.00	23.20	345.42	0.07
0.53	0	Calculated							

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Link-14 CHANNEL 0 02:32 0.67 1.00 22.95 191.95 0.12 0.29 0 Calculated

Link CSP-3 (41) Link CSP-2 (39) Link Link-04 (36) Link Link-08 (31) Link Link-09 (15)

Analysis began on: Sat Nov 20 15:07:47 2021
Analysis ended on: Sat Nov 20 15:07:49 2021

Autodesk® Storm and Sanitary Analysis 2016 - Version 13.4.121 (Build 0) Project Description File Name M20005-post-NOV9-21-1.SPF ****** Analysis Options Flow Units LPS Subbasin Hydrograph Method. SCS TR-20 Time of Concentration..... User-Defined Link Routing Method Hydrodynamic Storage Node Exfiltration.. None Starting Date NOV-09-2021 00:00:00 ***** Element Count ******* Number of rain gages 1 Number of subbasins 4 Number of nodes 16 Number of links 14 * * * * * * * * * * * * * * * * Raingage Summary ****** Gage Data Data Recording ID Source Type Interval min _____ Rain Gage-01 100 Year CUMULATIVE 6.00 ****** Subbasin Summary Subbasin Total Peak Rate Area Factor hectares -----5.53 484.00 1.24 484.00 4.16 484.00 0.32 484.00 Sub-01a Sub-01b Sub-02 Sub-04 ****** Node Summary ***** Node ID _____
 Jun-01
 JUNCTION
 198.90
 202.20
 0.00

 Jun-02
 JUNCTION
 201.48
 202.35
 0.00

 Jun-03
 JUNCTION
 201.55
 202.10
 0.00

 Jun-04
 JUNCTION
 201.75
 202.26
 0.00

 Jun-05
 JUNCTION
 202.33
 204.00
 0.00

Jun-06 Jun-07 Jun-08 Jun-09 Jun-10 Jun-11 Jun-12 Jun-13 Jun-14 Out-01 Out-02	JUNCTION OUTFALL	203.00 203.50 201.00 199.84 201.60 202.46 202.72 201.87 198.56	204.00 202.70 201.64 202.00 204.00 204.00 203.05 202.10 199.01	0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0		

Link ID	From Node	To Node	Element Type	Length m		Manning's Roughness
CSP-1	Jun-07	Jun-06	CONDUIT	8.0		0.0240
CSP-2	Jun-05	Jun-04	CONDUIT	8.0		0.0240
CSP-3	Jun-03	Jun-02	CONDUIT	8.0		0.0240
Link-01	Jun-01	Out-01	CONDUIT	19.0		0.0150
Link-02	Jun-08	Jun-01	CHANNEL	7.9		0.0300
Link-04	Jun-04	Jun-03	CHANNEL	20.0		0.0300
Link-06	Jun-06	Jun-05	CHANNEL	15.4		0.0300
Link-08	Jun-02	Jun-08	CHANNEL	26.1		0.0300
Link-09	Jun-09	Out-02	CONDUIT	11.5		0.0150
Link-10	Jun-10	Jun-09	CHANNEL	10.0		0.0320
Link-11	Jun-11	Jun-14	CHANNEL	63.2		0.0320
Link-12 Link-13	Jun-12 Jun-13	Jun-11 Jun-12	CONDUIT CHANNEL	8.7 21.0		0.0240 0.0320
Link-14	Jun-14	Jun-10	CHANNEL	5.5		0.0320
************* Cross Section ********** Link Design ID Flow	Summary	Depth/ Diameter	Width	No. of Barrels	Sectional	-
Capacity					Area	Radius
LPS		m	m		m²	m
CSP-1 386.59	CIRCULAR	0.45	0.45	1	0.16	0.11
CSP-2 415.85	CIRCULAR	0.45	0.45	1	0.16	0.11
CSP-3 144.47	CIRCULAR	0.45	0.45	1	0.16	0.11
Link-01 330.30	CIRCULAR	0.45	0.45	1	0.16	0.11
Link-02 299865.48	TRAPEZOIDAL	1.20	31.80	1	20.88	0.65
Link-04 1163.01	TRAPEZOIDAL	0.45	3.00	1	0.84	0.27
Link-06 7562.79	TRAPEZOIDAL	1.00	2.75	1	1.75	0.49
Link-08 1351.97	TRAPEZOIDAL	0.26	7.24	1	1.07	0.15
Link-09 421.21	CIRCULAR	0.50	0.50	1	0.20	0.12

Link-10	TRAPEZOIDAL	0.20	1.95		1 0.27	7 (
5.98 Link-11	TRAPEZOIDAL	0.20	3.40		1 0.44	<u> </u>
7.34 Link-12 .17	CIRCULAR	0.30	0.30		1 0.07	7 (
.17 Link-13 5.42	TRAPEZOIDAL	0.20	3.00		1 0.40) (
Link-14 1.95	TRAPEZOIDAL	0.10	2.20		1 0.16	5 (
*****	*****	Volume	Depth			
Runoff Quant:	ity Continuity ******	hectare-m	mm			
Surface Runoi	itation ff rror (%)	0.926 0.955 -0.003	82.309 84.947			
Flow Routing	************* Continuity *******	Volume hectare-m	Volume Mliters			
External Infi External Out Initial Store Final Stored	low flow ed Volume Volume rror (%)	0.000 0.331 0.000 0.003 0.004	0.000 3.311 0.000 0.030			
Subbasin Sub-	-01a					
Soil/Surface				Area	Soil	
- Jomposito Am	Description			Area (ha)	Soil Group	CN
composite Are	Description			(ha)	Group	
Subbasin Sub-	Description ea & Weighted CN			(ha) 5.53	Group	78.00
Subbasin Sub-	Description ea & Weighted CN 01b			(ha) 5.53	Group	78.00
Subbasin Sub- Soil/Surface	Description ea & Weighted CN -01b Description			(ha) 5.53 5.53	Group 	78.00 78.00
Subbasin Sub- Soil/Surface	Description ea & Weighted CN Description ea & Weighted CN -02			(ha) 5.53 5.53 Area (ha) 1.24	Group 	78.00 78.00 78.00
Subbasin Sub- Soil/Surface Composite Are Subbasin Sub-	Description ea & Weighted CN -01b Description ea & Weighted CN -02			(ha) 5.53 5.53 Area (ha) 1.24	Group 	78.00 78.00 78.00
Subbasin Sub- Soil/Surface Composite Are Subbasin Sub-	Description ea & Weighted CN -01b Description ea & Weighted CN -02 Description			(ha) 5.53 5.53 Area (ha) 1.24 1.24 Area	Group Soil Group - Soil	78.00 78.00 78.00
Subbasin Sub	Description ea & Weighted CN 01b Description ea & Weighted CN Description ea & Weighted CN 02 Description			(ha) 5.53 5.53 Area (ha) 1.24 1.24 Area (ha) 4.16	Group Soil Group Soil Group	78.00 78.00 78.00 CN 76.60 76.60
Subbasin Sub	Description ea & Weighted CN -01b Description ea & Weighted CN 02 Description ea & Weighted CN -04			(ha) 5.53 5.53 Area (ha) 1.24 1.24 Area (ha) 4.16	Group Soil Group Soil Group	78.00 78.00 78.00 CN 76.60 76.60

- 0.32 - 83.40 Composite Area & Weighted CN 0.32 - 83.40

Subbasin ID	Total Precip mm	Total Runoff mm	Peak Runoff LPS	Weighted Curve Number	Time of Concentration days hh:mm:ss
Sub-01a	82.70	33.38	378.32	78.000	0 00:16:42
Sub-01b	82.70	31.17	87.22	76.600	0 00:08:05
Sub-02	82.70	26.87	203.32	73.700	0 00:21:20
Sub-04	82.70	42.80	34.83	83.400	0 00:01:38

Node ID	Average Depth Attained	Maximum Depth Attained	Maximum HGL Attained		of Max irrence	Total Flooded Volume	Total Time Flooded	Retention Time
	m	m	m	days	hh:mm	ha-mm	minutes	hh:mm:ss
Jun-01	0.44	1.11	200.01	0	02:40	0	0	0:00:00
Jun-02	0.04	0.07	201.55	0	02:35	0	0	0:00:00
Jun-03	0.14	0.29	201.84	0	02:35	0	0	0:00:00
Jun-04	0.06	0.12	201.87	0	02:35	0	0	0:00:00
Jun-05	0.13	0.26	202.59	0	02:33	0	0	0:00:00
Jun-06	0.04	0.08	203.08	0	02:34	0	0	0:00:00
Jun-07	0.00	0.00	203.50	0	00:00	0	0	0:00:00
Jun-08	0.43	0.50	201.50	0	02:35	0	0	0:00:00
Jun-09	0.25	0.51	200.35	0	02:40	0	0	0:00:00
Jun-10	0.01	0.03	201.63	0	02:32	0	0	0:00:00
Jun-11	0.04	0.07	202.53	0	02:31	0	0	0:00:00
Jun-12	0.11	0.20	202.68	0	02:30	0	0	0:00:00
Jun-13	0.03	0.06	202.78	0	02:30	0	0	0:00:00
Jun-14	0.02	0.04	201.91	0	02:31	0	0	0:00:00
Out-01	0.21	0.45	199.01	0	02:32	0	0	0:00:00
Out-02	0.14	0.26	199.91	0	02:40	0	0	0:00:00

Node ID	Element Type	Maximum Lateral Inflow LPS	Peak Inflow LPS	Peak Occu	ime of Inflow rrence hh:mm	Maximum Flooding Overflow LPS	Time of Peak Flooding Occurrence days hh:mm
Jun-01	JUNCTION	374.94	452.80	0	02:36	0.00	
Jun-02	JUNCTION	0.00	84.58	0	02:35	0.00	
Jun-03	JUNCTION	0.00	85.11	0	02:35	0.00	
Jun-04	JUNCTION	0.00	86.50	0	02:33	0.00	
Jun-05	JUNCTION	0.00	84.74	0	02:35	0.00	
Jun-06	JUNCTION	84.76	84.76	0	02:34	0.00	
Jun-07	JUNCTION	0.00	0.00	0	00:00	0.00	
Jun-08	JUNCTION	0.00	84.53	0	02:35	0.00	
Jun-09	JUNCTION	200.44	226.04	0	02:39	0.00	
Jun-10	JUNCTION	0.00	32.41	0	02:31	0.00	

Jun-11	JUNCTION	0.00	32.51	0	02:30	0.00
Jun-12	JUNCTION	0.00	32.69	0	02:30	0.00
Jun-13	JUNCTION	32.71	32.71	0	02:30	0.00
Jun-14	JUNCTION	0.00	32.44	0	02:31	0.00
Out-01	OUTFALL	0.00	440.62	0	02:40	0.00
Out-02	OUTFALL	0.00	225.82	0	02:40	0.00

Outfall Node ID	Flow	Average	Peak
	Frequency	Flow	Inflow
	(%)	LPS	LPS
Out-01	94.99	159.58	440.62
Out-02	94.86	89.20	225.82
System	94 92	248 78	665 94

Link ID Ratio of		Element Stal Reported	Т	ime of	Maximum	Length	Peak Flow	Design	Ratio of
		Type	Pea	k Flow	Velocity	Factor	during	Flow	Maximum
Maximum	num Time Condition		Occu	rrence	Attained		Analysis	Capacity	/Design
Flow Surc	harged		davs	hh:mm	m/sec		LPS	LPS	Flow
Depth	minutes	;			, 222				
CSP-1 0.09	0	CONDUIT Calculated	0	00:00	0.00	1.00	0.00	386.59	0.00
CSP-2	0	CONDUIT	0	02:33	1.37	1.00	86.50	415.85	0.21
0.42 CSP-3	0	Calculated CONDUIT	0	02:35	1.51	1.00	84.58	144.47	0.59
0.40	0	Calculated	U	02.35	1.51	1.00	04.50	144.47	0.59
Link-01	21	CONDUIT	0	02:40	2.77	1.00	440.62	330.30	1.33
Link-02	21	CHANNEL	0	02:35	0.03	1.00	84.51	299865.48	0.00
	0		Λ	02:35	0.35	1 00	85 11	1163 01	0 07
0.45	0	Calculated							
	0		0	02:35	0.54	1.00	84.74	7562.79	0.01
Link-08		CHANNEL	0	02:35	0.22	1.00	84.53	1351.97	0.06
	0		0	02:40	1.41	1.00	225.82	421.21	0.54
0.76	0	Calculated							
	0		0	02:32	0.25	1.00	32.41	926.98	0.03
Link-11		CHANNEL	0	02:31	0.43	1.00	32.44	337.34	0.10
	0		0	02:30	1.05	1.00	32.51	25.17	1.29
0.45	0	> CAPACITY							
Link-13 0.64	0	CHANNEL Calculated	0	02:30	0.16	1.00	32.69	345.42	0.09
Link-01 1.00 Link-02 0.47 Link-04 0.45 Link-06 0.17 Link-08 0.64 Link-09 0.76 Link-10 0.58 Link-11 0.28 Link-12 0.45 Link-13	21 0 0 0 0 0 0	CONDUIT SURCHARGED CHANNEL Calculated CHANNEL Calculated CHANNEL Calculated CHANNEL Calculated CHANNEL Calculated CONDUIT Calculated CHANNEL Calculated CHANNEL Calculated CHANNEL Calculated CHANNEL Calculated CHANNEL Calculated CHANNEL CAPACITY CHANNEL	0 0 0 0 0	02:35 02:35 02:35 02:35 02:40 02:32	0.03 0.35 0.54 0.22 1.41	1.00 1.00 1.00 1.00 1.00	84.51 85.11 84.74 84.53 225.82 32.41	299865.48 1163.01 7562.79 1351.97 421.21 926.98	0.00 0.07 0.01 0.06 0.54

Link-14 CHANNEL 0 02:31 0.74 1.00 32.41 191.95 0.17 0.36 0 Calculated

Link CSP-2 (40) Link Link-04 (38) Link CSP-3 (38) Link Link-08 (24) Link Link-09 (20)

Analysis began on: Fri Nov 19 17:07:11 2021
Analysis ended on: Fri Nov 19 17:07:12 2021

Drawings

